



**STUDIO TECNICO PROF. L.M. NERI**

FORMAZIONE • SICUREZZA • ANTINCENDIO  
SISTEMI DI GESTIONE • SVILUPPO ORGANIZZATIVO  
POTENZIAMENTO GRUPPI DI LAVORO E PERSONALE

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**EVENTI SISMICI: PREVENZIONE, PROTEZIONE,  
SICUREZZA, EMERGENZA**

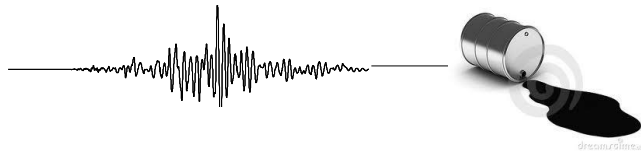
**XVI Incontro LA SICUREZZA DEL LAVORO NEL XXI SECOLO:  
QUALI PROSPETTIVE? Ricordo del Prof. Ing. Werther Neri**



# La Vulnerabilità e Protezione Sismica di Componenti di Impianti Industriali

*Fabrizio Paolacci*

*Università degli Studi Roma Tre*

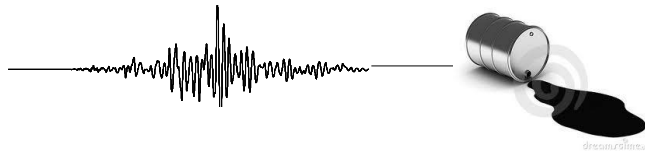


**Novembre 17 2016, Bologna**

# Outline

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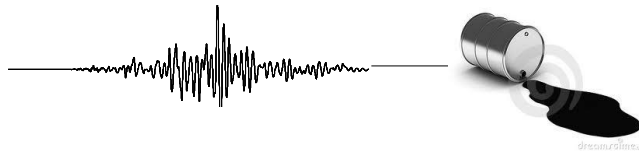
- Introduction
- Definition of the problem
- Structural classification of Industrial equipment and typical damages under seismic action
- Assessment methods for plant components: storage tanks
- Example: Seismic assessment of an elevated tank
- Example: Base Isolation of an elevated tank



# Objectives

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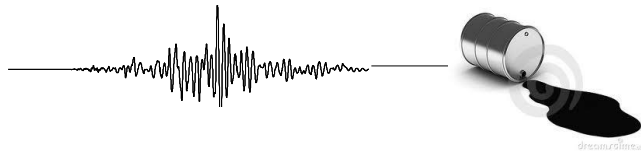
- ❑ Identification of the main industrial components (under structural point of view)
- ❑ Collection into a limited number of classes based of geometrical and mechanical characteristics
- ❑ Synthesis of the effects of earthquakes on the identified structural typologies of process components
- ❑ Description of assessment methods for storage tanks
- ❑ Analysis of a Case study
- ❑ Applicability of base isolation of the seismic protection of storage tanks



# Definition of the problem

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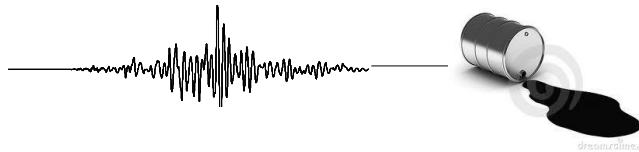
- Among events that can cause serious accidents to industrial plants, seismic action (**NATECH**) must be potentially considered one of the most important. As a matter of fact, in Italy about 30% of industrial plants with major-accident hazards are located in areas with a high seismic risk.
- In addition, in case of a seismic event, the earthquake can induce the simultaneous damage of more apparatus, which effects can result amplified because of the unsuccessful working of safety systems or because of the simultaneous generation of multiple accidental chains.



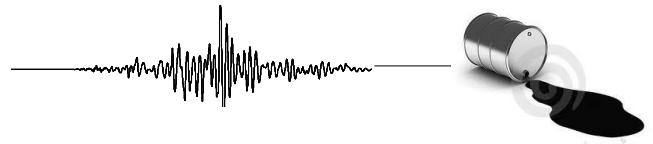
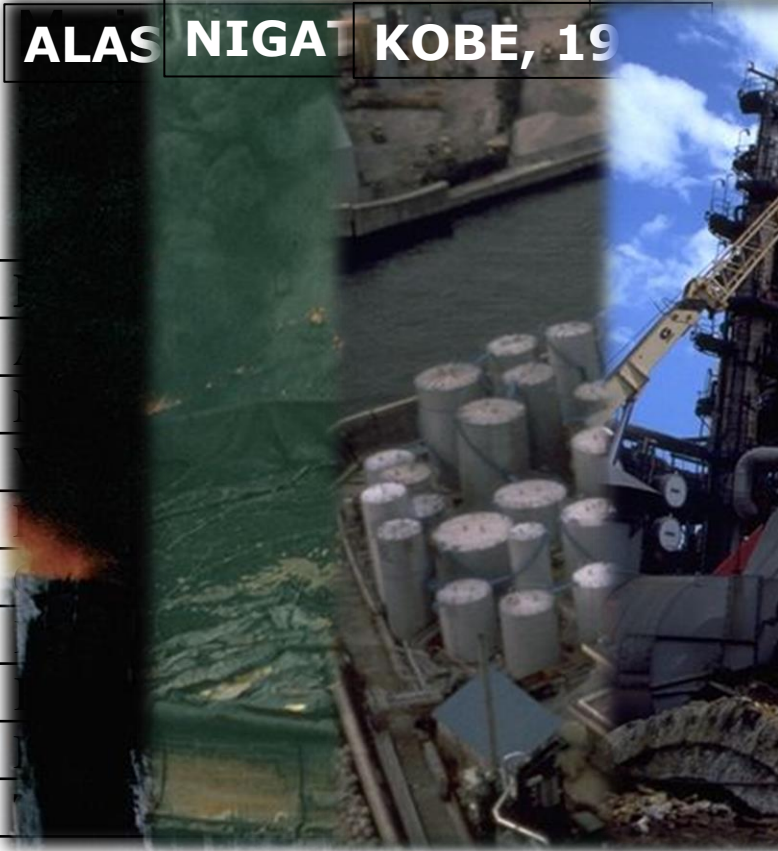
# Definition of the problem

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- The usual safety requirements applied to civil buildings (ultimate and serviceability limit states) are generally unsuitable for structures of industrial plants.
- As a matter of fact, a critical damage for a process safety that can cause an even modest release of inflammable substances, such as opening a flange or breaking a welding, or the simple friction between floating roof and tanks can result unessential from the structural damage point of view, but, at the same time, can cause considerable accidental chains.



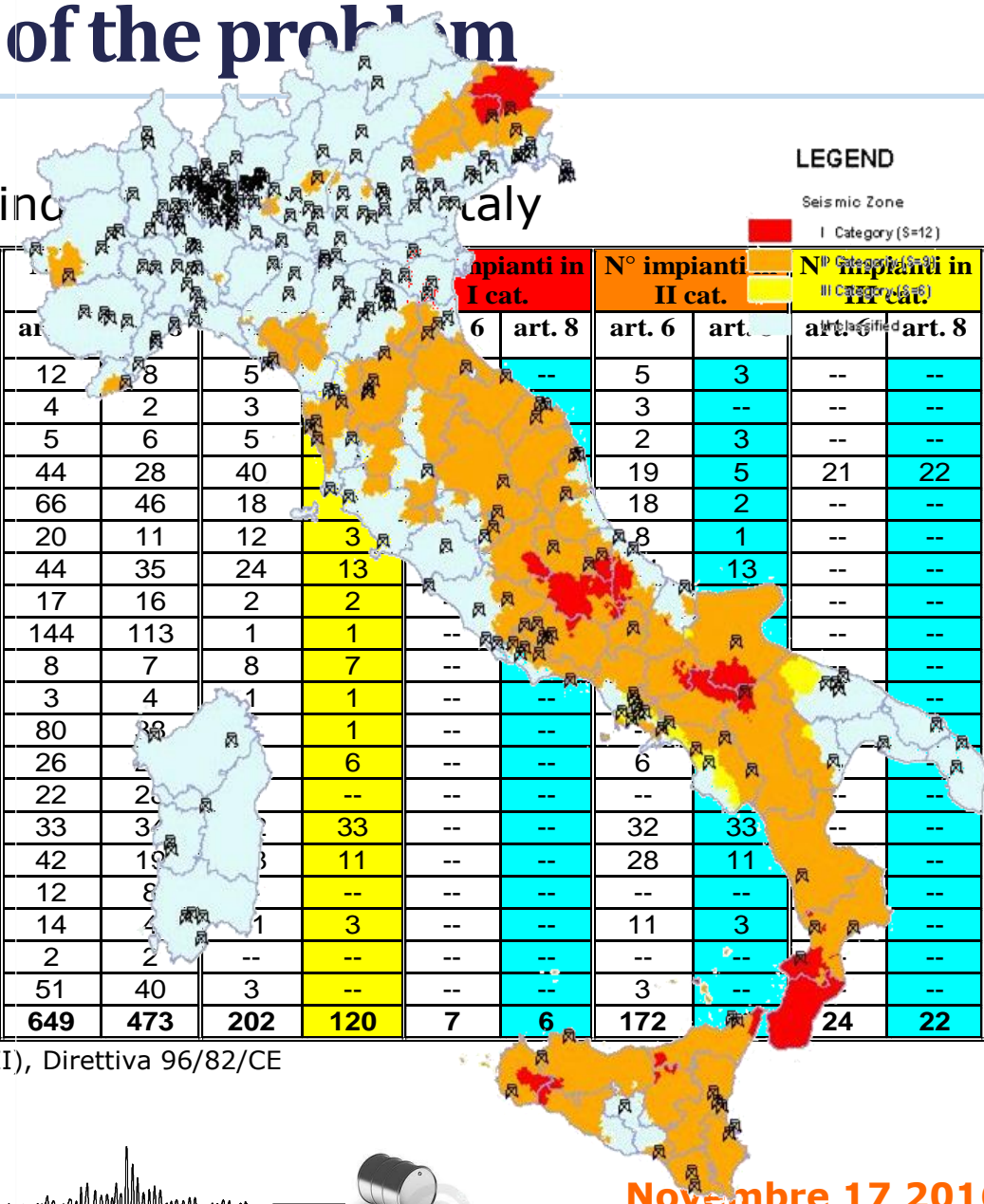
# Definition of the problem





# Definition of the problem

## Major Hazard in Italy



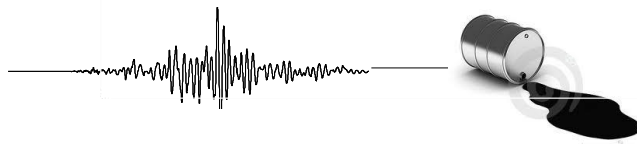
### LEGEND

Seismic Zone

I Category (S=12)

| REGIONE             | I cat.     |            | II cat.    |            | III cat.   |           | % impianti in zona sismica |              |
|---------------------|------------|------------|------------|------------|------------|-----------|----------------------------|--------------|
|                     | art. 6     | art. 8     | art. 6     | art. 8     | art. 6     | art. 8    | art. 6                     | art. 8       |
| ABRUZZO             | 12         | 8          | 5          | --         | 5          | 3         | 41,7%                      | 37,5%        |
| BASILICATA          | 4          | 2          | 3          | --         | 3          | --        | 75%                        | 50%          |
| CALABRIA            | 5          | 6          | 5          | --         | 2          | 3         | 100%                       | 100%         |
| CAMPANIA            | 44         | 28         | 40         | 3          | 19         | 5         | 90,9%                      | 96,4%        |
| EMILIA-ROMAGNA      | 66         | 46         | 18         | 1          | 18         | 2         | 27,3%                      | 4,3%         |
| FRIULI-VEN.GIULIA   | 20         | 11         | 12         | 3          | 8          | 1         | 60%                        | 27,3%        |
| LAZIO               | 44         | 35         | 24         | 13         | 44         | 13        | 54,5%                      | 37,1%        |
| LIGURIA             | 17         | 16         | 2          | 2          | --         | --        | 11,8%                      | 12,5%        |
| LOMBARDIA           | 144        | 113        | 1          | 1          | --         | --        | 0,7%                       | 0,9%         |
| MARCHE              | 8          | 7          | 8          | 7          | --         | --        | 100%                       | 100%         |
| MOLISE              | 3          | 4          | 1          | 1          | --         | --        | 33,3%                      | 25%          |
| PIEMONTE            | 80         | 38         | 1          | 1          | --         | --        | --                         | 2,6%         |
| PUGLIA              | 26         | 26         | 6          | 6          | 26         | 6         | 34,6%                      | 25%          |
| SARDEGNA            | 22         | 22         | --         | --         | --         | --        | --                         | --           |
| SICILIA             | 33         | 34         | 33         | 33         | 33         | 33        | 97%                        | 97,1%        |
| TOSCANA             | 42         | 19         | 3          | 11         | 28         | 11        | 66,7%                      | 57,9%        |
| TRENTINO-ALTO ADIGE | 12         | 8          | --         | --         | --         | --        | --                         | --           |
| UMBRIA              | 14         | 4          | 1          | 3          | 11         | 3         | 78,6%                      | 75%          |
| VAL D'AOSTA         | 2          | 2          | --         | --         | --         | --        | --                         | --           |
| VENETO              | 51         | 40         | 3          | --         | 3          | --        | 5,9%                       | --           |
| <b>ITALIA</b>       | <b>649</b> | <b>473</b> | <b>202</b> | <b>120</b> | <b>172</b> | <b>22</b> | <b>31,1%</b>               | <b>25,4%</b> |

D. Lgs. 334/99 (Seveso II), Direttiva 96/82/CE

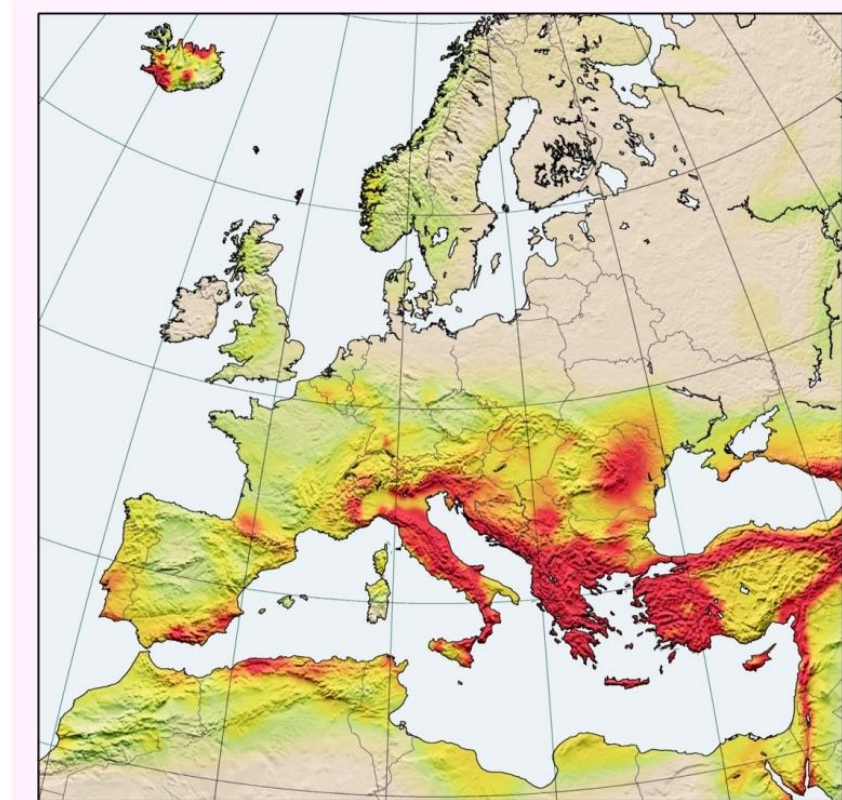
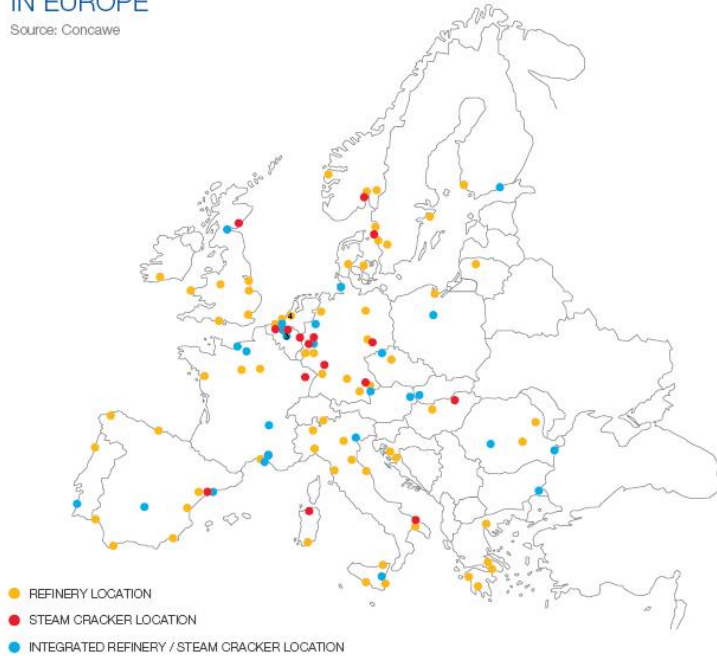


# Definition of the problem

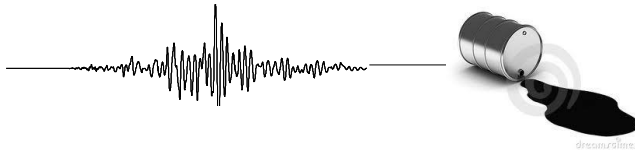
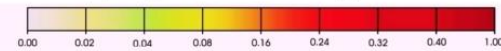
## Refinery plants in Europe

### REFINERY/STEAM CRACKER SITES IN EUROPE

Source: Concawe



peak ground acceleration (g)

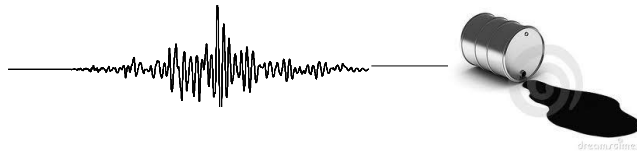




# Equipment of a petrochemical plant

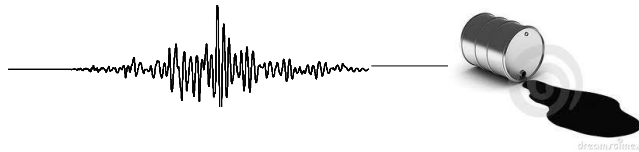


Process flow of a Petrochemical plant  
(Moulein & Makkee, 1987)



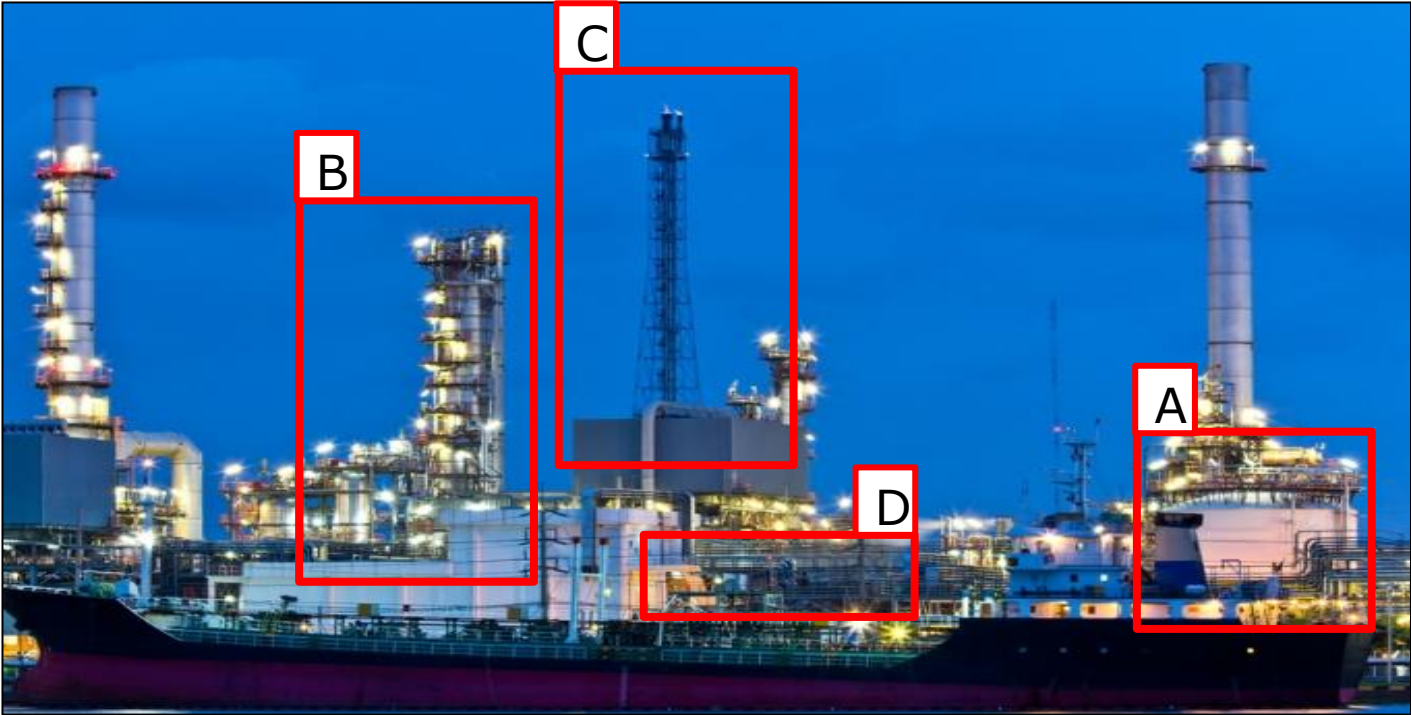
# Equipment of a petrochemical plant

## Typical layout of a Petroleum Refinery



# Equipment of a petrochemical plant

## Main components of a Petroleum refinery



- A** Storage Tanks
- B** Process Equipment
- C** Torches and flares
- D** Pipelines



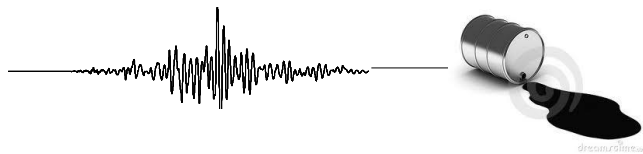


# Structural classification and typical seismic damages

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- Slim vessels
- Squat equipment directly placed on the foundation
- Squat equipment supported by columns
- Piping, pipelines and supporting structures

F. Paolacci, R. Giannini, M. De Angelis, (2013), Seismic response mitigation of chemical plant components by passive control systems, *Journal of Loss Prevention in Process Industries*, Volume 26, Issue 5, Pages 879-948 Special Issue: Process Safety and Globalization - **DOI:10.1016/j.jlp.2013.03.003.**



# Structural classification: SLIM VESSELS

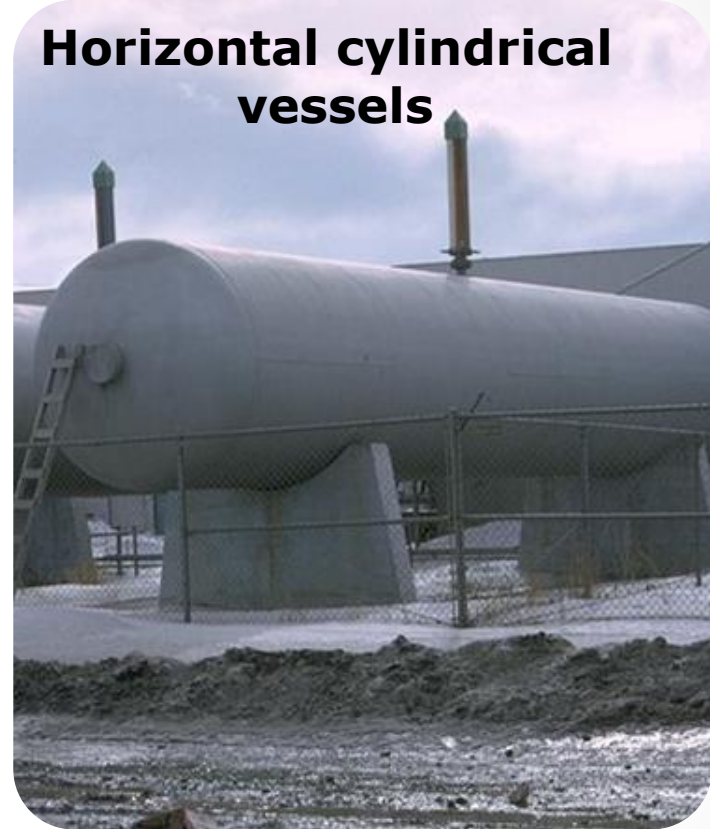
**Distillation columns  
and reactors**



**Stacks,  
Torches  
Flares**



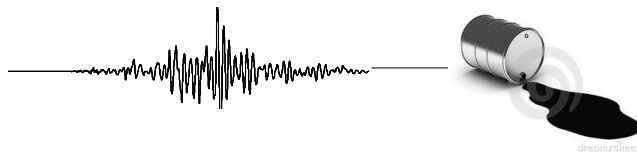
**Horizontal cylindrical  
vessels**





# Structural classification: SLIM VESSELS (typical damages)

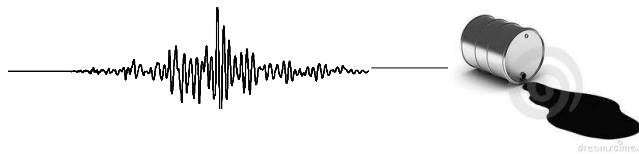
- the most common damages are the



# Structural classification:

## Squat eq. directly placed on the foundation

- These apparatus are characterized by heavy masses;
- the main category of structures belonging to this group is the large cylindrical steel storage tanks with a height/diameter ratio between 0.2 and 2. The roof can be welded to the shell (fixed conic roof) or floating over the contained liquid. The operating volume varies from some tens to 200000 m<sup>3</sup>.



# Structural classification:

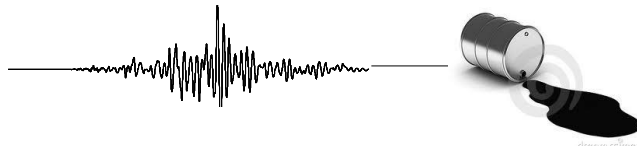
## Squat eq. directly placed on the foundation (Typical damages)

- The relative buckling of both
- Other sloshing when cracked
- Up



Structures are  
phantom foot  
the tank  
excessive  
ating roof,  
due to the

**(possible damages due to floating roof sloshing, Tokachi-oki 2003)**



# Structural classification: Squat eq. supported by columns

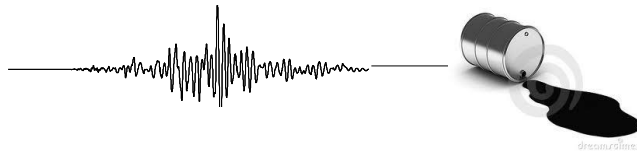


Spherical storage vessels

Cylindrical elevated tanks



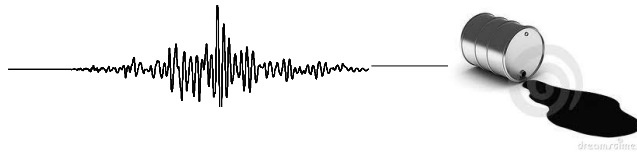
Cylindrical or Cathedral-type Heaters





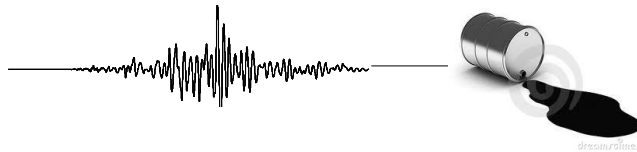
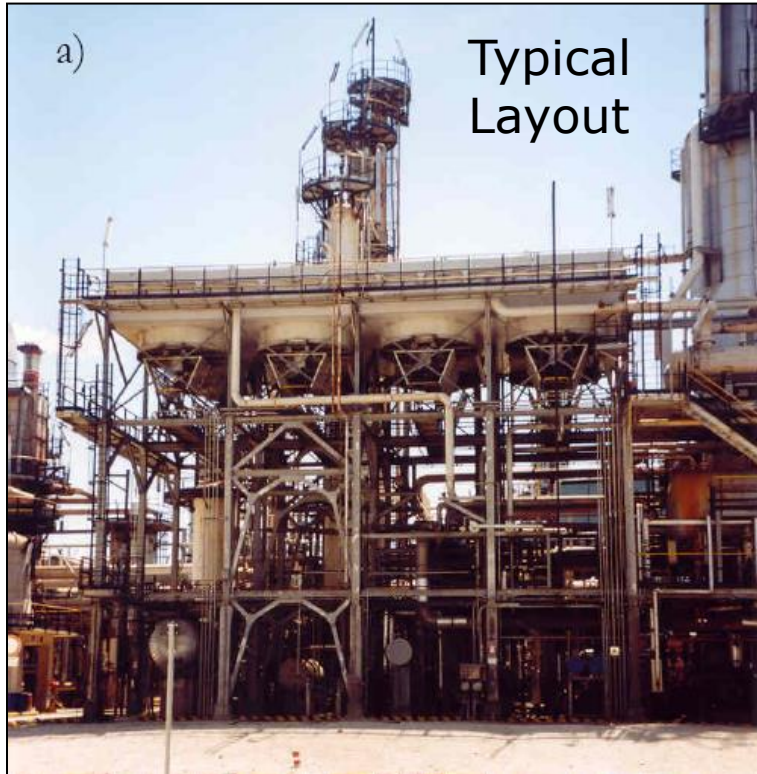
# Structural classification: Squat eq. supported by columns (typical damages)

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# Structural classification: Pipes, pipelines and support structure



# Structural classification: Pipes, pipelines and support structure



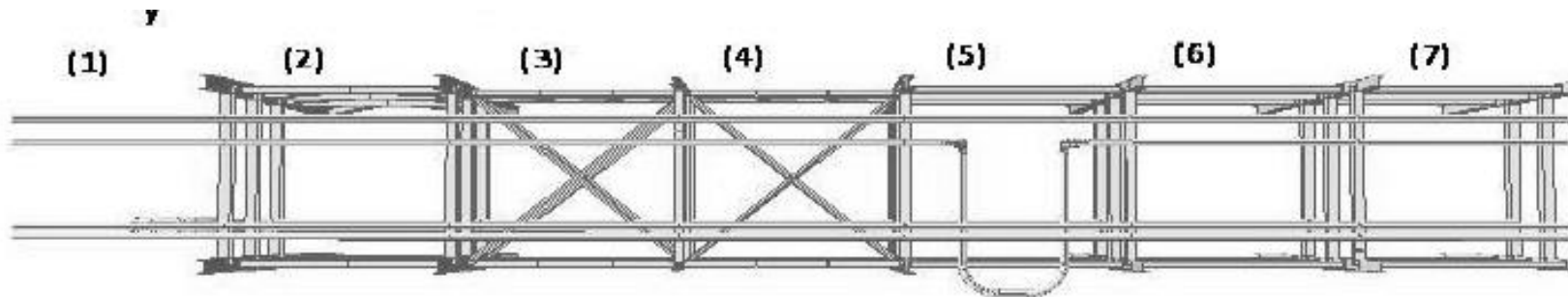
Flange Bolted Joints



Elbows

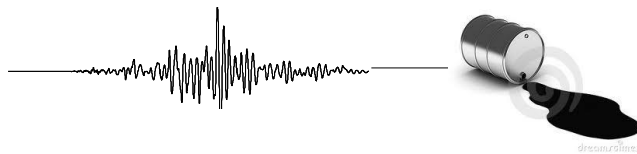


Tee-joints



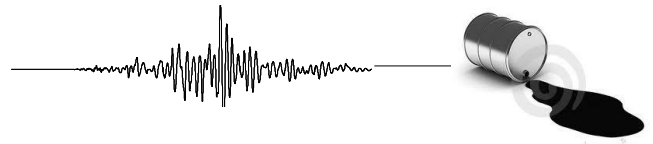
# Typological and damage-based classification of plant components

| Structural typology                            | Critical equipment  | Typical seismic observed damages   | Other possible damages   |
|--|---|--|--|
| <b>Slim vessels</b>                            | <b>Columns<br/>Reactors<br/>Chimney<br/>Torch</b>                                   | <ul style="list-style-type: none"> <li>• Leakage of fluid in flanged joints</li> <li>• Yielding of anchor bars</li> </ul>  | <b>Overtipping</b>   |
| <b>Above-ground squat equipment</b>            | Big broad tanks with fixed and floating roof  | <b>Failure of wall-bottom plate welding</b><br><b>Elephant foot buckling</b><br><b>Diamond buckling of tank wall</b><br><b>Settlements of ground</b><br><br><b>Impact of floating roof to tank wall.</b>   | <b>Uplifting</b><br><br><br><br><br><br><b>Overtopping</b><br><b>Torch fire</b>  |
| <b>Squat equipment placed on short columns</b> | <b>Spherical tanks</b><br><br><b>Process Furnaces</b><br><br><b>Cryogenic tanks</b> | <b>Collapse of structure due to shear failure of columns</b><br><br><b>Collapse of structure due to shear failure of columns</b><br><b>Collapse of the chimney</b><br><b>Detachment of internal pipes</b><br><b>Detachment of the internal refractory material</b><br><b>Collapse of structure due to shear failure of columns</b> | <b>Leakage from pipes;</b><br><br><b>Increase of temperature of Furnace wall</b> |
| <b>Piping systems and support structure</b>    | <b>Steel or R.C. frames</b>   | <b>Collapse for excessive stresses</b>   | <b>Damages to supported equipment (pipes, tanks,..)</b>                          |





# Seismic assessment methods for plant components storage tanks



# Seismic assessment methods for plant components storage tanks

Limit States for tanks  
Typologies of tanks





# Seismic assessment methods for plant components

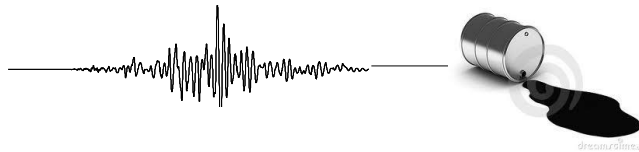
## storage tanks

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The assessment of seismic vulnerability of storage tanks, is usually performed using the well-know concept of Fragility: the probability of exceeding of a certain limit state,  $P(D > LS | PGA)$ . It is evaluated for different values of selected Intensity Measures. A suitable IM for tanks often adopted in literature is the PGA.

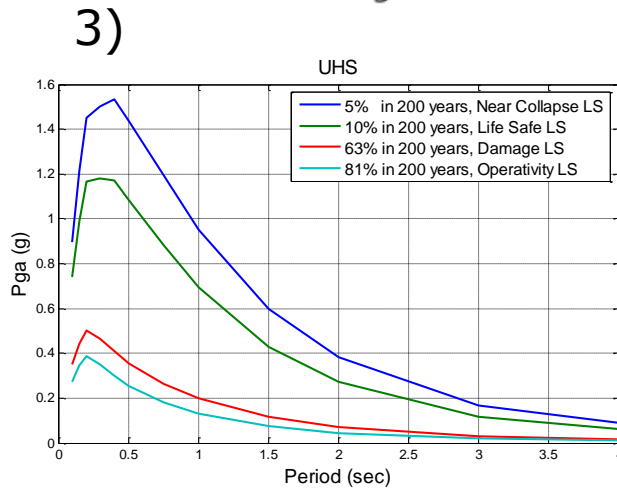
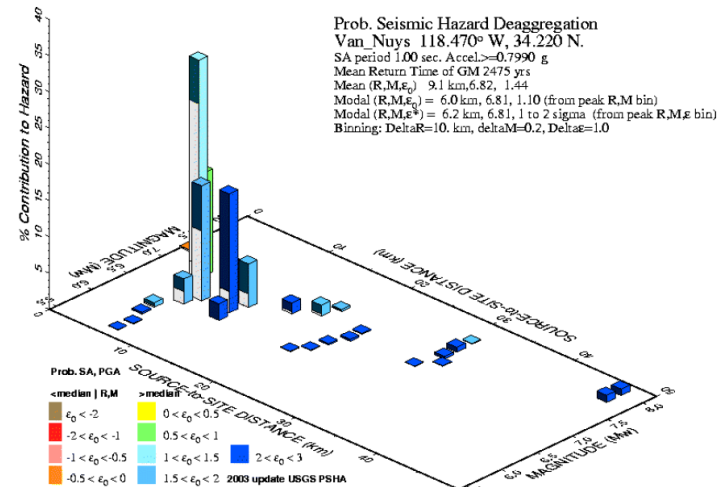
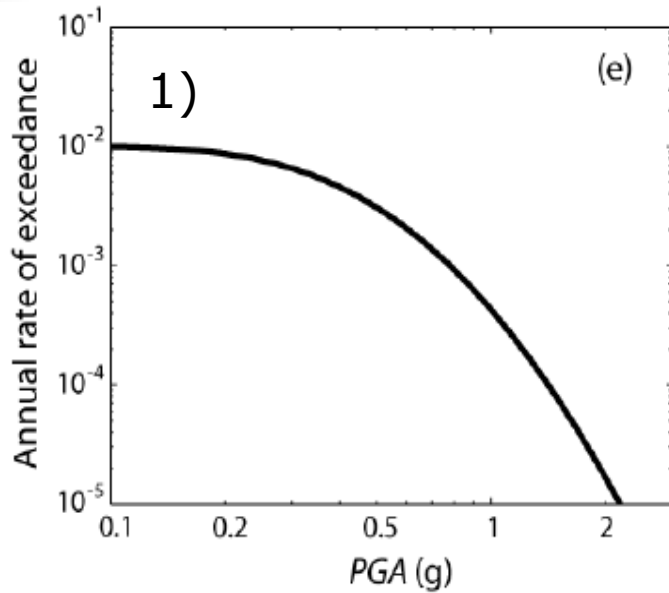
The main steps to evaluate fragility curves are the following:

- Definition of hazard and input signals
- Definition of Dynamic models
- Definition of Limit States
- Calculation of probability of failure

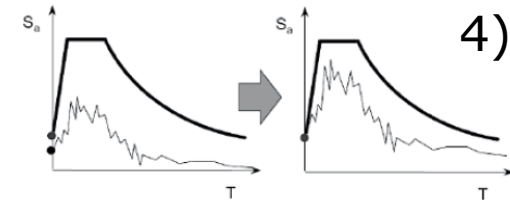


# Assessment methods for plant components: storage tanks

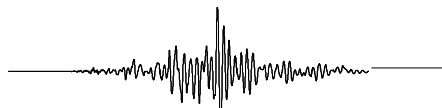
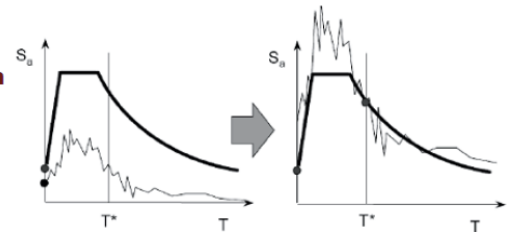
## 1. Seismic Hazard and input signals



Match to Peak Ground Acceleration (PGA)



Match Spectral Acceleration at Specific Period



# Assessment methods for plant components: storage tanks

## 2. Dynamic modeling

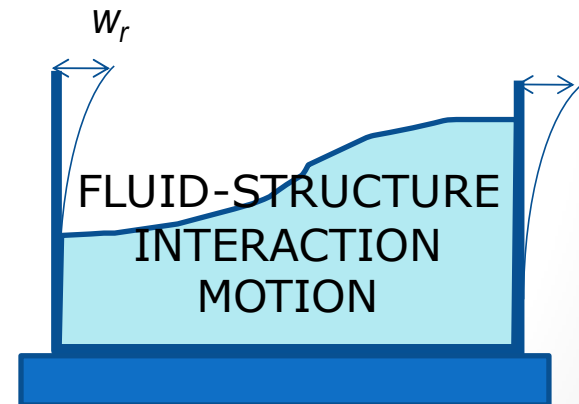
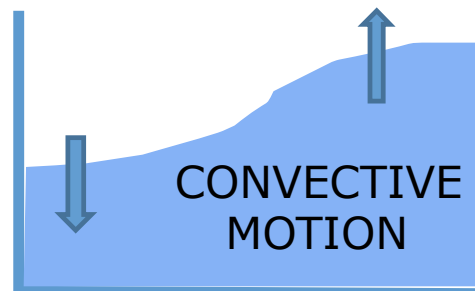
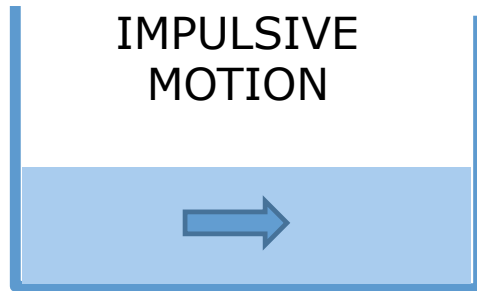
### Hypotesis

- “Perfect Liquid “ (unviscous and incompressible)
- Laminar and slow motion
- Free liquid surfaces

$$\mathbf{v} = \text{grad}(\phi)$$

Laplace Equation

$$\Delta\phi = \frac{\partial^2\phi}{\partial x^2} + \frac{\partial^2\phi}{\partial y^2} + \frac{\partial^2\phi}{\partial z^2} = 0$$

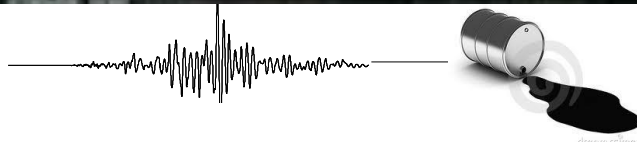
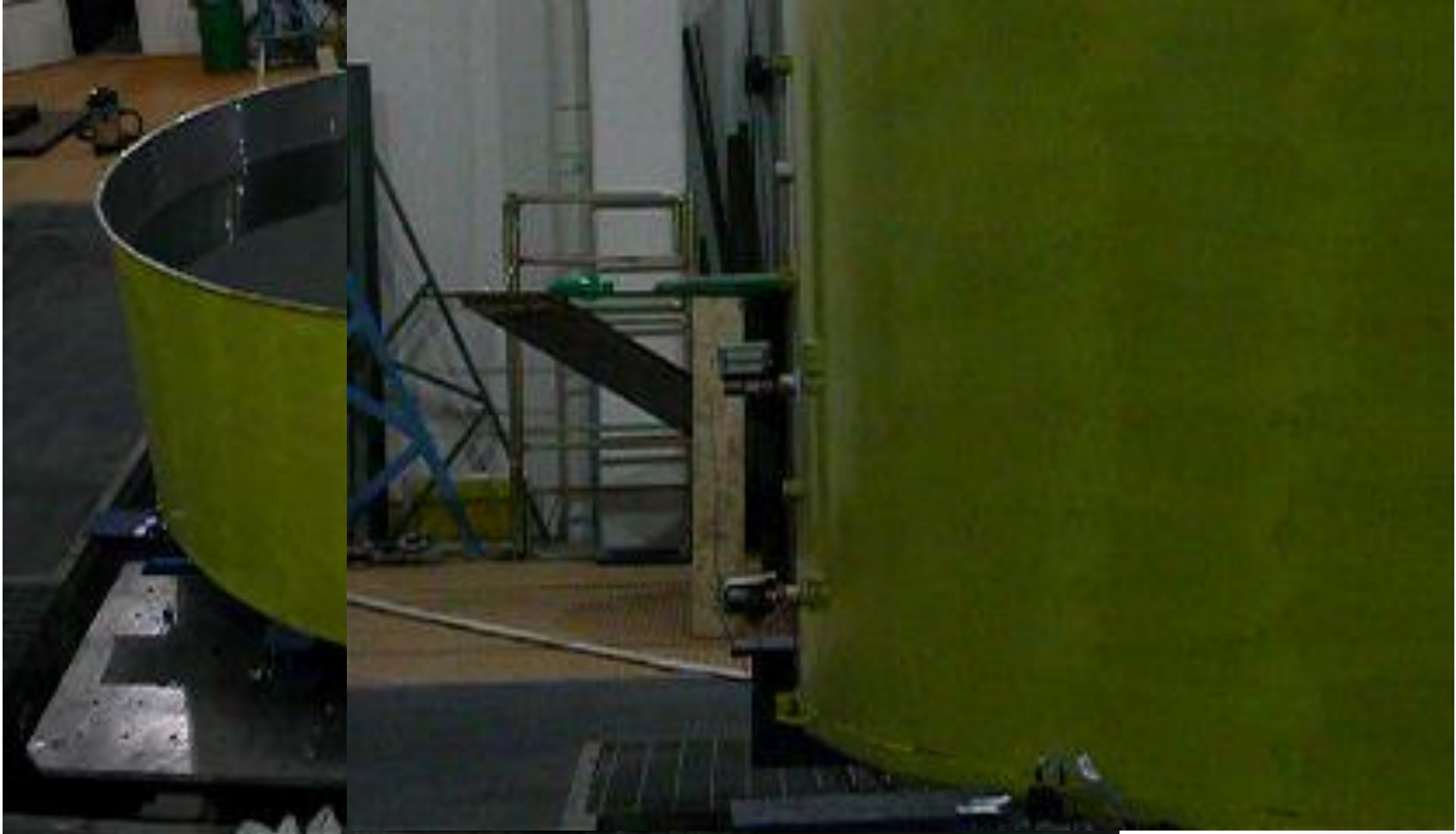


ground motion

# Assessment methods for plant components: storage tanks

## 2. Dynamic modeling

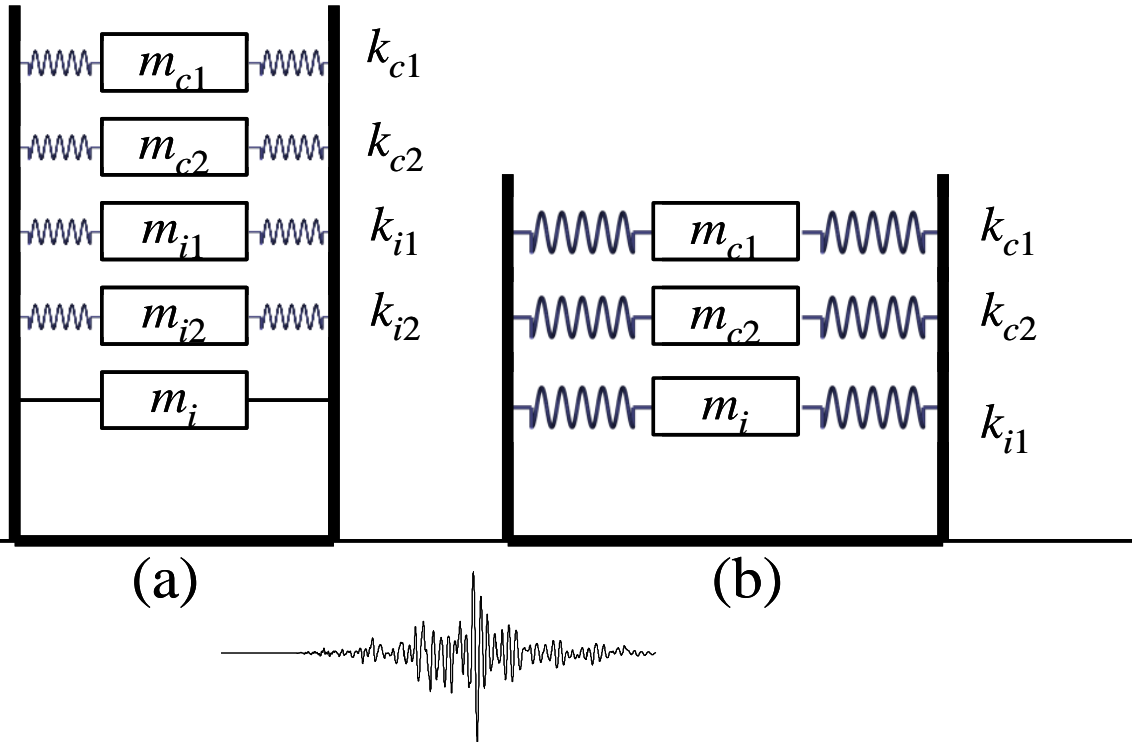
M. De Angelis, R. Giannini, F. Paolacci, (2010), Experimental investigation on the seismic response of a steel liquid storage tank equipped with floating roof by shaking table tests, *Earthquake Engineering & Structural Dynamics*, 39: 377–396. DOI: 10.1002/eqe.945



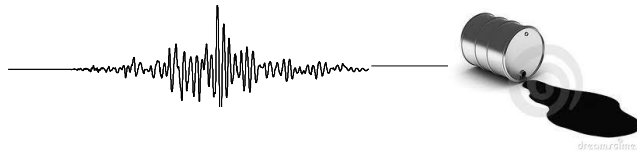


# Assessment methods for plant components: storage tanks

## 2. Dynamic modeling

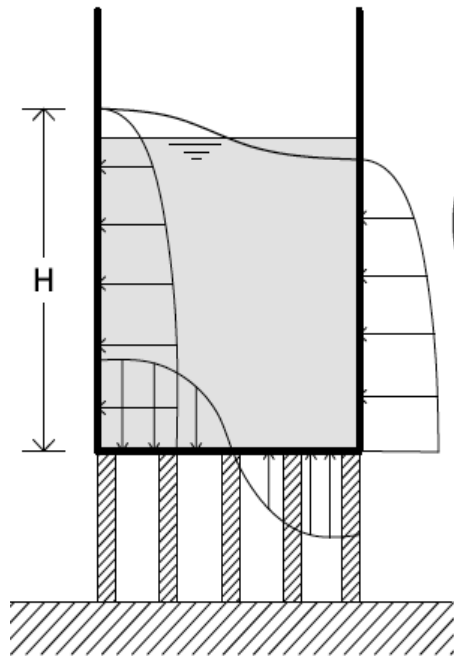


**Lumped mass models of tanks: (a) Slender, (b) Broad**

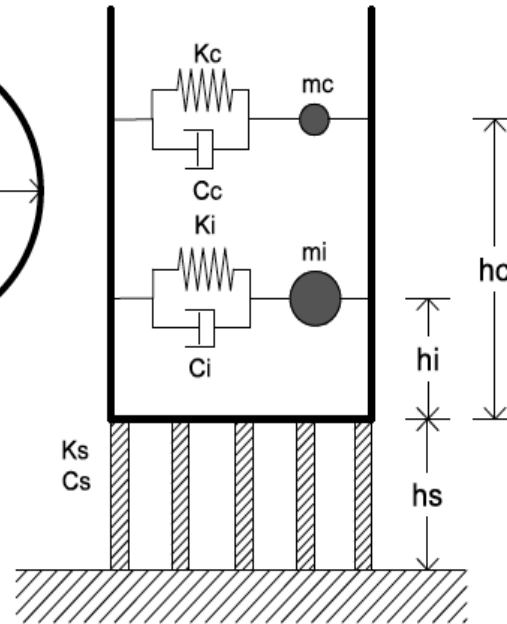
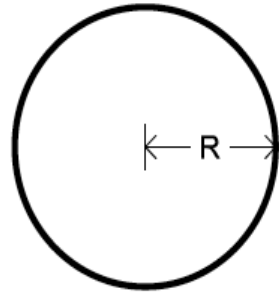


# Assessment methods for plant components: storage tanks

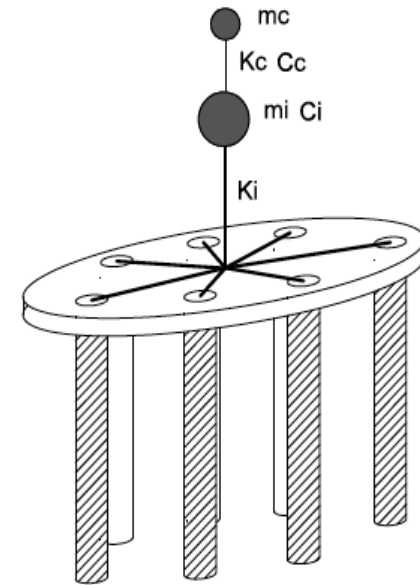
## 2. Dynamic modeling



(a)

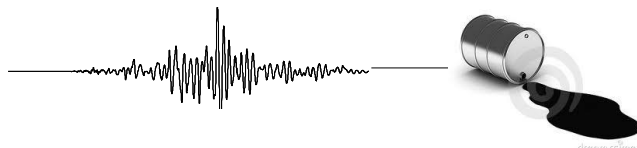


(b)



(c)

### Lumped mass models of elevated tanks



# Assessment methods for plant components: storage tanks

## 3. Definition of limit states

- Damages are in the form of cracking at the corner of the bottom plate and buckling of tank wall due to uplift, sliding of the base, anchorage failure, sloshing damage around the roof, failure of piping systems and plastic deformation of base plate.



**ELASTOPLASTIC BUCKLING  
(Elephant's Foot)**



**ELASTIC BUCKLING  
(Diamond shape)**

# Assessment methods for plant components: storage tanks

## 3. Definition of limit states

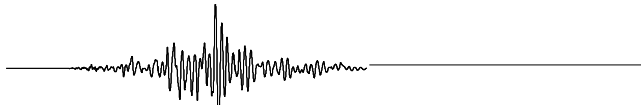
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**OVERTOPPING**



**UPLIFTING**





## 3. Definition of limit states

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Each Damage State can be quantified analytically using for example the analytical formula provided by the current regulations. Buckling problems have been well solved in literature whose solutions are included in most of the standards and codes. For example:

### Elephant foot buckling (EN1998:4)

$$f_{pb} = \sigma_{c1} \cdot \left[ 1 - \left( \frac{p \cdot R}{t_w \cdot f_y} \right)^2 \right] \cdot \left( 1 - \frac{1}{1.12 + r^{1.5}} \right) \cdot \left( \frac{r + f_y/250}{r + 1} \right),$$

where  $\sigma_{c1} = 0.605 \frac{E_w t_w}{R}$  is the Euler's critical axial compressive stress,  $R$  is the tank radius,  $p$  is the total internal pressure,  $E_w$  and  $t_w$  are the elastic modulus and the thickness of the tank walls,  $f_y$  is the steel yielding stress, and  $r$  is a coefficient defined as  $r = R/(400 \cdot t)$ .



# Assessment methods for plant components: storage tanks

## 4. Calculation of Fragility curves

- The structural fragility can be defined as the probability of exceeding a selected Limit State (LS) for a specified the Intensity Measure (IM).
- A lognormal cumulative distribution function is often used to define a fragility function:

$$P[D_{EDP} > LS | IM] = \Phi \left( \frac{\ln(IM / \mu)}{\beta} \right)$$

probability of exceeding a selected  $LS$  for a specified  $IM$

median of the fragility function

standard deviation of  $\ln IM$  (dispersion of  $IM$ )



# Assessment methods for plant components: storage tanks

## 4. Calculation of Fragility curves

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- Non-linear dynamic analysis procedures → relationship between EDP and IM:
  - Cloud Method (Bazzurro et al. 1998; Luco and Cornell 1998; Jalayer 2003)
  - Incremental Dynamic Analysis (IDA) (Vamvatsikos and Cornell 2002)
  - Multiple-Stripe Analysis (MSA) (Bazzurro et al. 1998; Baker 2007; Jalayer and Cornell 2009)



# Assessment methods for plant components: storage tanks

## 4. Calculation of Fragility curves: **Cloud Analysis**

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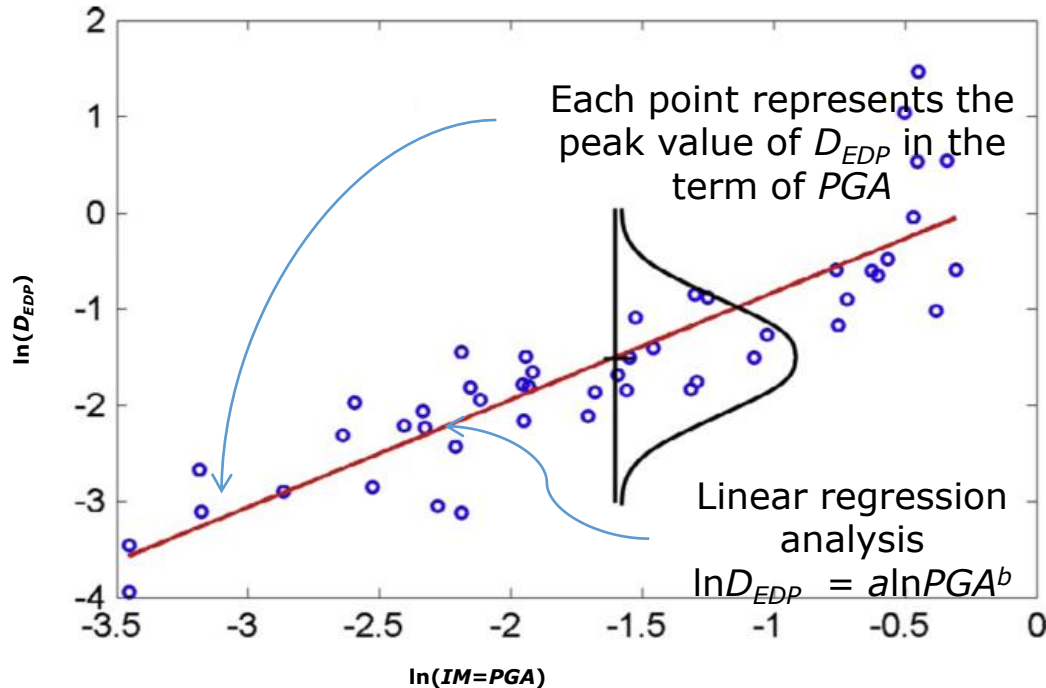
- Cloud Analysis method uses a set of un-scaled ground motion records.
- This method implements the non-linear dynamic analysis results in a (linear) regression-based probabilistic model.
- Assumption of a constant conditional standard deviation for probability distribution of the EDP given IM.
- Strong dependence on the suite of ground motion records.





# Assessment methods for plant components: storage tanks

## 4. Calculation of Fragility curves: **Cloud Analysis**



### Example: Cloud Analysis results

$$D_m = a(IM)^b$$

mean

$a$  and  $b$  are regression coefficients based on the collection of  $d_i$  and  $IM_i$ .

$$\beta_{d|IM} = \sqrt{\frac{\sum_{i=1}^n [\ln(d_i) - \ln(aIM_i^b)]^2}{n-2}}$$

Dispersion



# Assessment methods for plant components: storage tanks

## 4. Calculation of Fragility curves : **Cloud Analysis**

- When the seismic demands and the structural limit states are assumed to follow a lognormal distribution, the probability of exceeding a specific damage state can be given as:

$$P[D_{EDP} > LS | IM] = 1 - \Phi \left( \frac{\ln(LS_m) - \ln(D_m)}{\sqrt{\beta_{d|IM}^2 + \beta_{LS}^2}} \right)$$

|                |  |
|----------------|--|
| $\Phi$         | standard normal cumulative distribution function |
| $LS_m$         | median estimate of the structural limit state    |
| $D_m$          | median estimate of the demand                    |
| $\beta_{d IM}$ | dispersion of the demand conditioned on the IM   |
| $\beta_{LS}$   | dispersion of the structural limit state         |

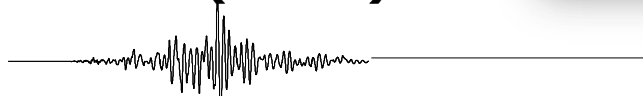


# Assessment methods for plant components: storage tanks

## **Example: Fragility curves evaluation of an LNG tank**



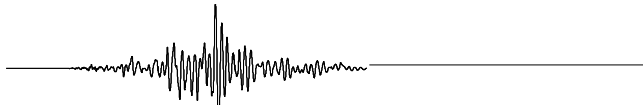
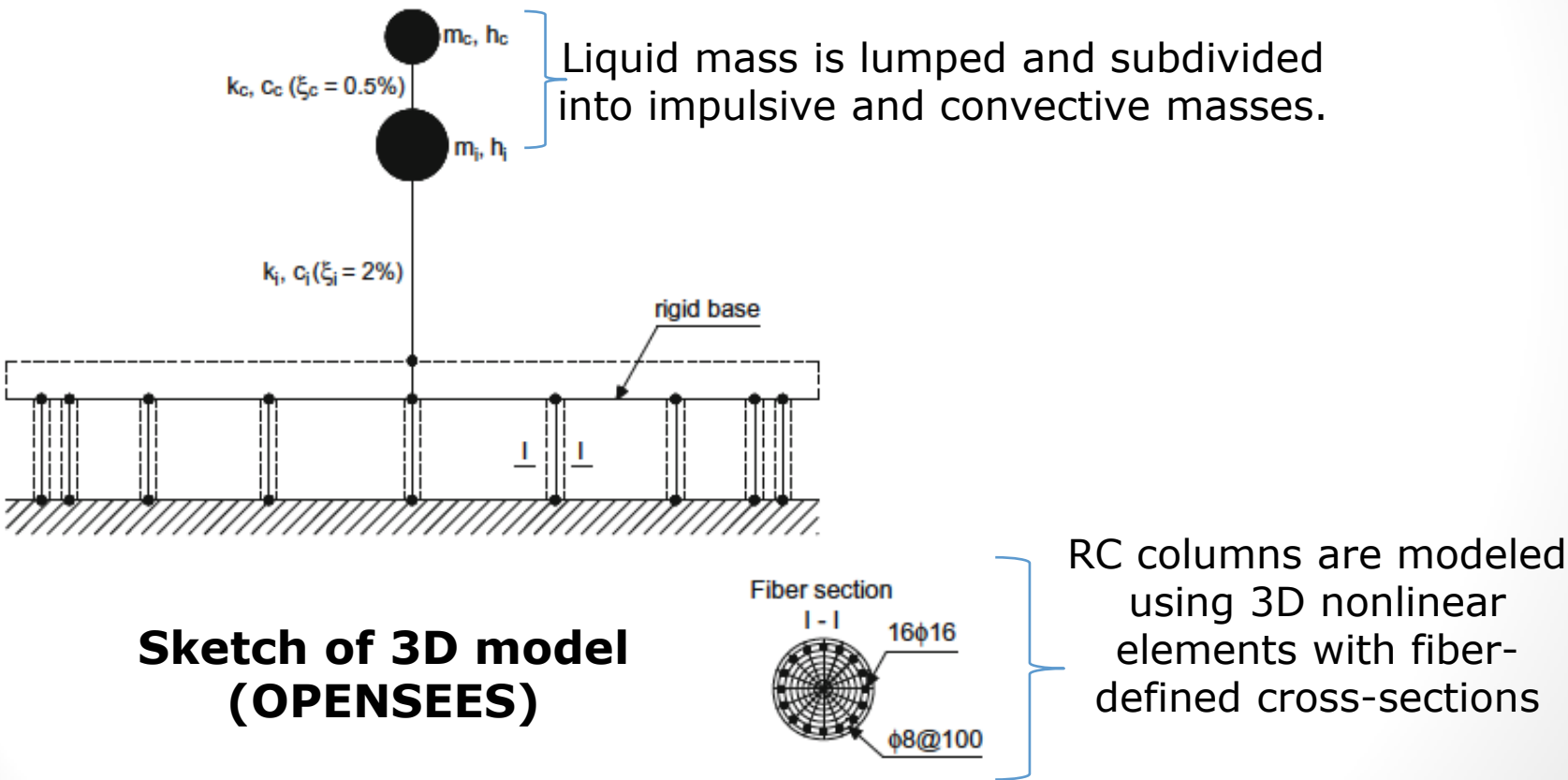
**Storage tanks of Liquid Oxygen at Habas plant after the strong event of Itzmit (1999)**



# Assessment methods for plant components: storage tanks

## Example: Fragility curves evaluation of an LNG tank

- 3D nonlinear modeling

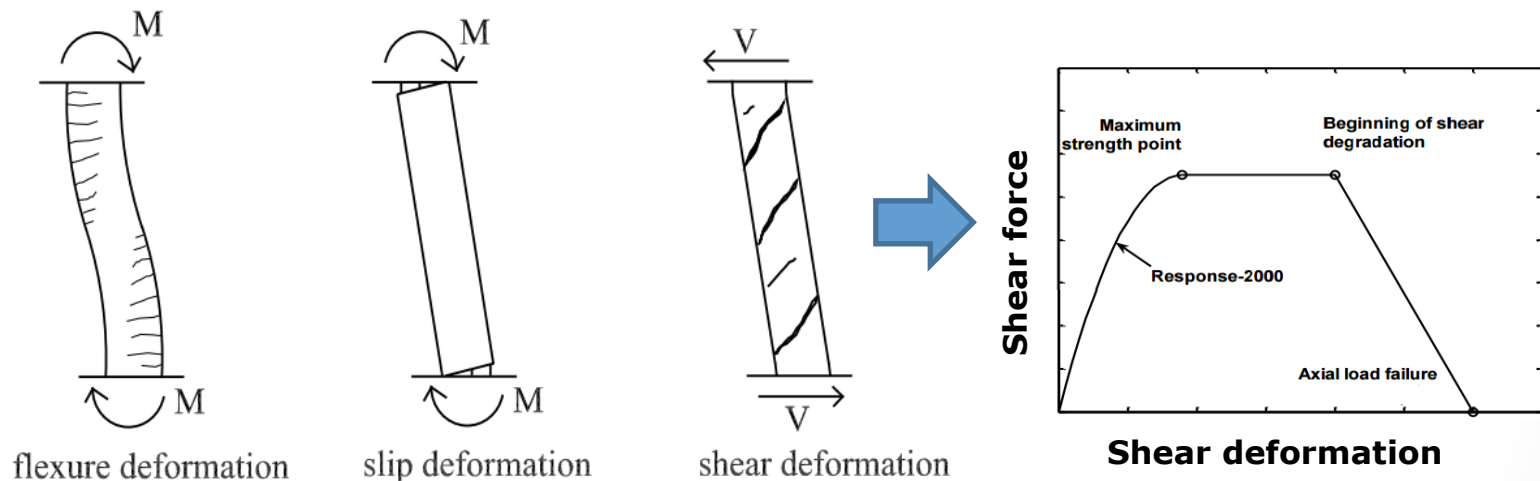




# Assessment methods for plant components: storage tanks

## Example: Fragility curves evaluation of an LNG tank

- Columns with small aspect ratio or without adequate shear-resisting reinforcement, shear deformation governs the total response → unexpected shear or shear-flexure failure.

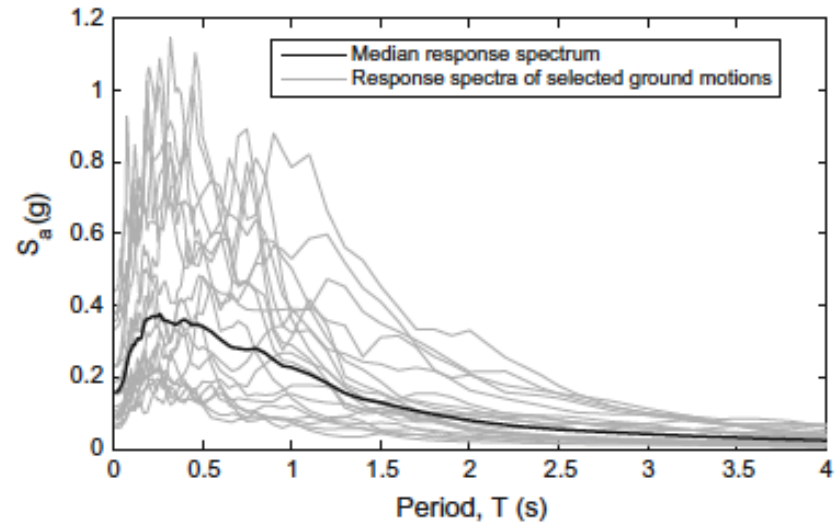


**The total lateral deformation of a fixed-ended RC column: flexural, reinforcement slip, and shear deformations.**

# Assessment methods for plant components: storage tanks

## Example: Fragility curves evaluation of an LNG tank

- The system was subjected to 20 natural records selected from PEER Strong Ground Motion:
  - Database Magnitude:  $5 < M < 7$
  - Distance from the fault:  $0 < d < 20$  km
  - S-waves velocity between 360 m/s and 760 m/s



**Response spectra of  
the 20 unscaled  
accelerograms**



# Assessment methods for plant components: storage tanks

## Example: **Fragility curves evaluation of an LNG tank**

---

- **Engineering Demand Parameters (EDP):**

- Drift ratio  $q = \frac{d}{H_{col}}$

- Compressive meridional stress (API 650)

$$S_z = \frac{1}{t_s} \left( w_t (1 + 0.4 a_{gv}) + 1.273 \frac{M_T}{D^2} \right)$$

- Elevation of liquid free surface (EN1998:4)

$$d_{\max} = 0.84 R S_a (T_c) / g$$



# Assessment methods for plant components: storage tanks

## Example: Fragility curves evaluation of an LNG tank

- **Limit states:**

- Ultimate Drift ratio at pure-shear failure

$$q_{LS} = 0.245\%$$

(\*)

- Drift ratio for pure flexural failure  
(Chord Rotation)

$$q_{LS} = 0.360\%$$

EN 1998-4

- Meridional design buckling stress (EN 1998-4)

$$\sigma_{x,Rd} = \sigma_{x,Rk} / \gamma_{M1} = (\chi_x \sigma_y) / \gamma_{M1}$$

- Free board height:

$$d_{LS} = H_{tank} - H_{liquid}$$

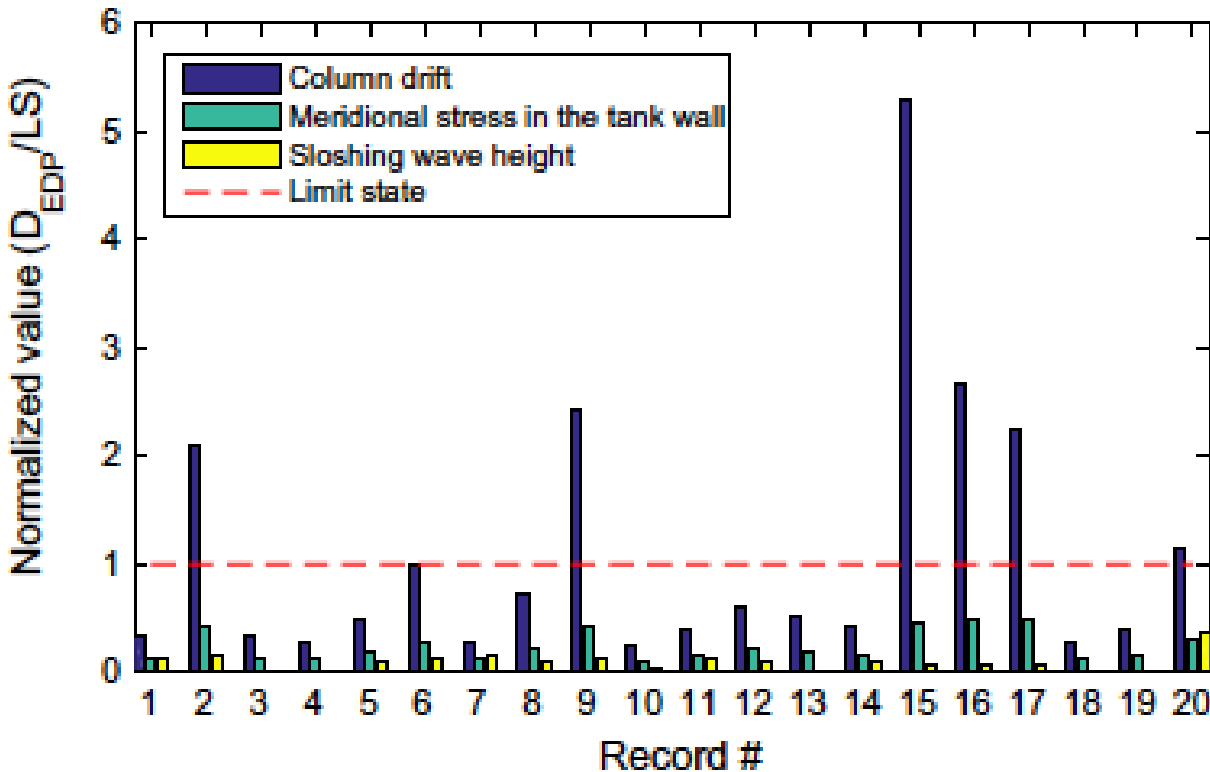
(\*) M. Gerin and P. Adebar (2004) **Accounting for Shear in Seismic Analysis of Concrete Structures**  
13th World Conference on Earthquake Engineering (WCEE),  
Vancouver, Canada, August 2004



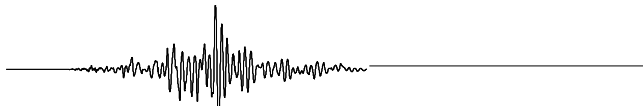


# Assessment methods for plant components: storage tanks

## Example: Fragility curves evaluation of an LNG tank



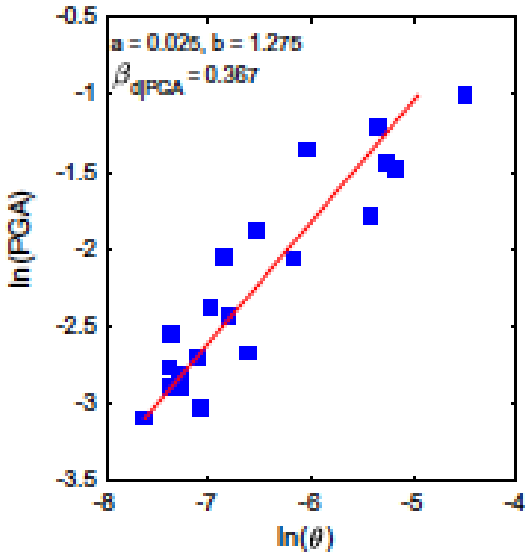
Seismic Demand Parameters



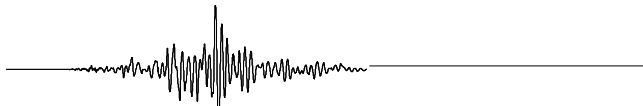
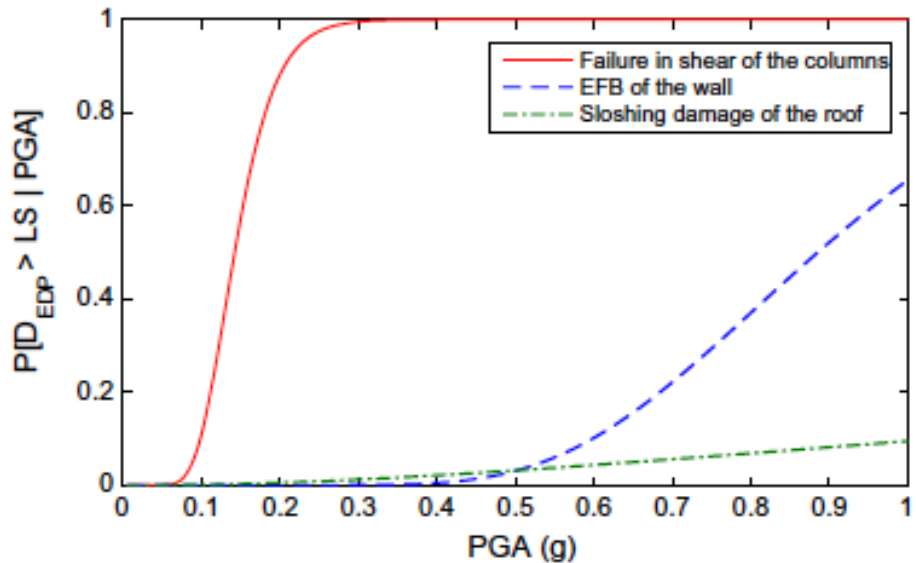
# Assessment methods for plant components: storage tanks

## Example: Fragility curves evaluation of an LNG tank

### Probabilistic Seismic Response Analysis (Cloud Analysis)

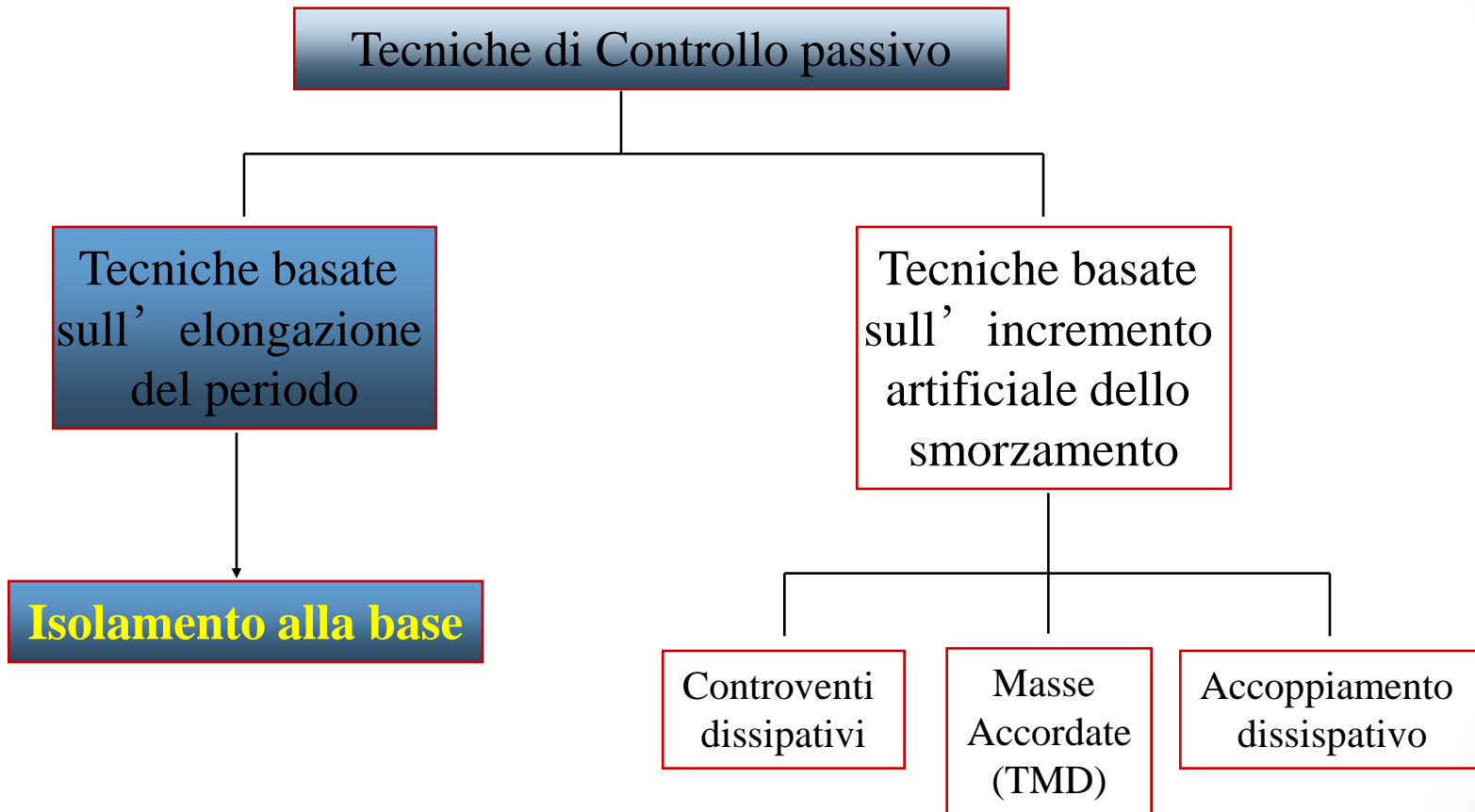


### Fragility curve for shear failure of columns



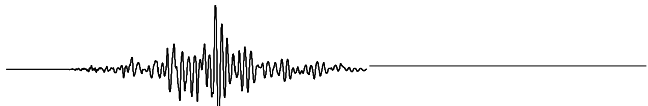
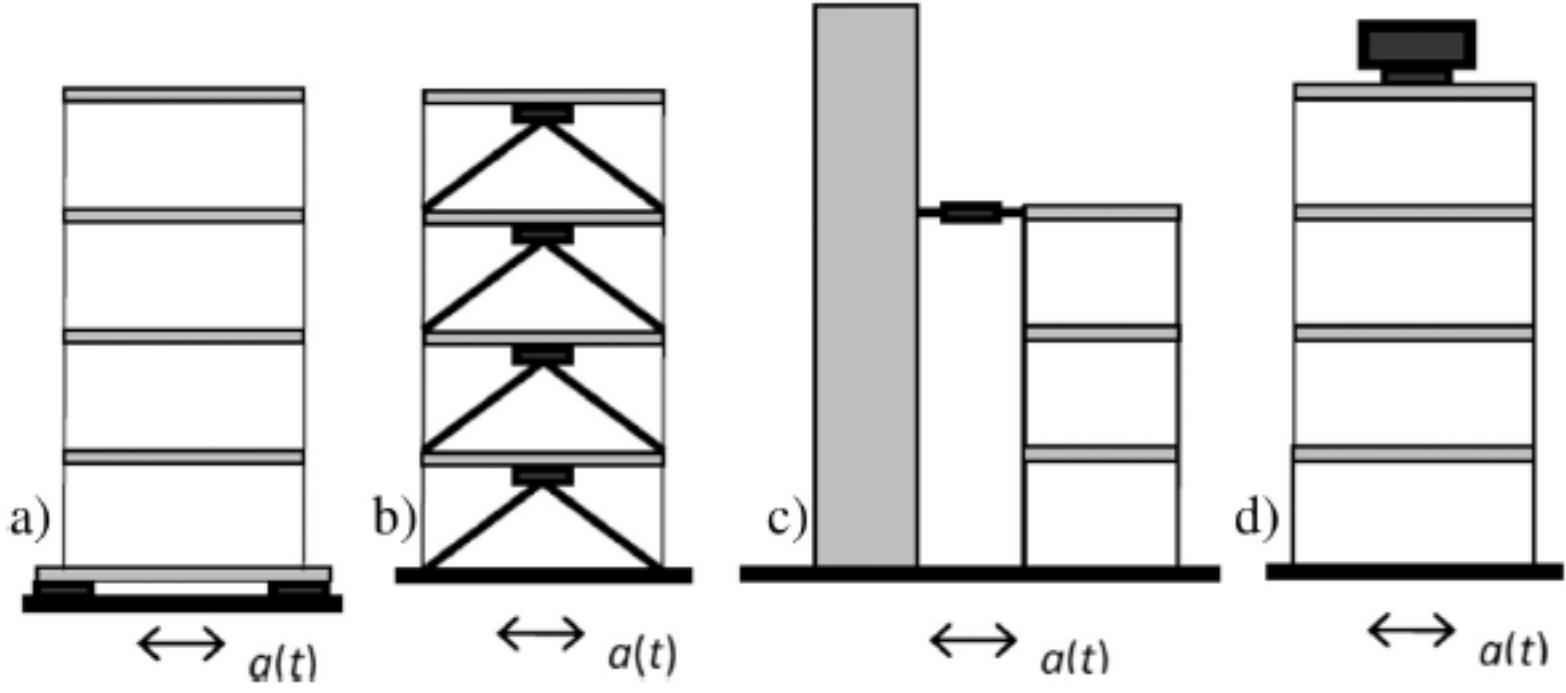
# Seismic protection of Industrial Components

## Passive Control Systems



# Seismic protection of Industrial Components

## Passive Control Systems

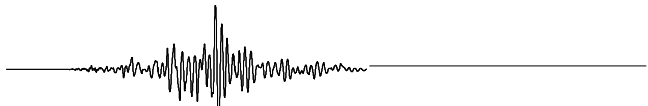
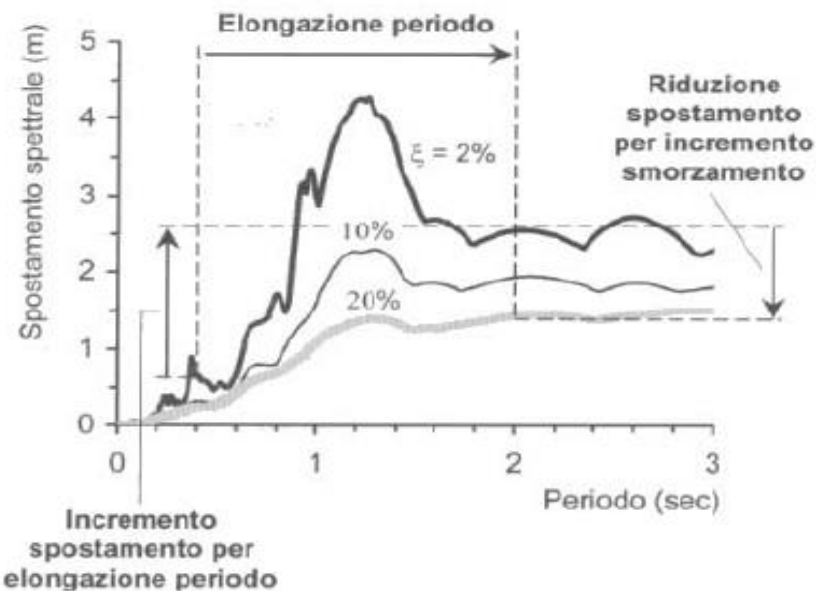
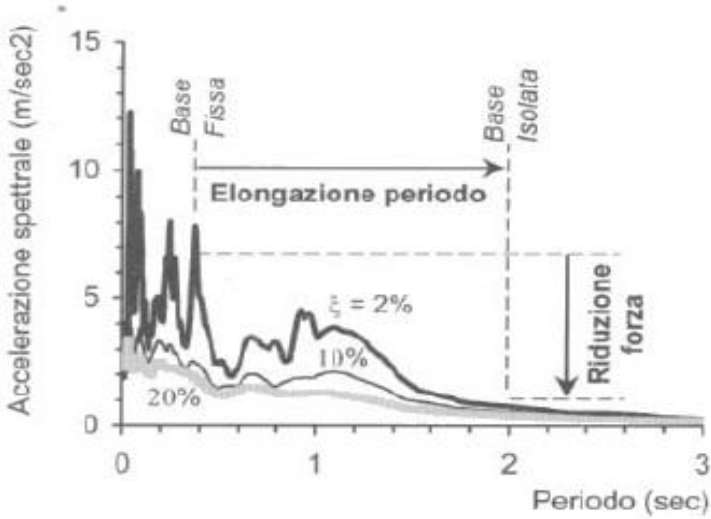


# Seismic protection of Industrial Components

## Passive Control Systems

### Principi di funzionamento dei SCP

Dolce ed al. “ progetti di edifici isolati alla base” ,  
IUSS Press





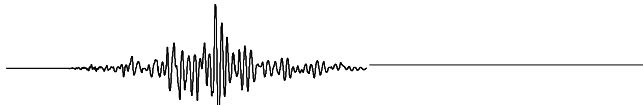
# Seismic protection of Industrial Components

## Passive Control Systems

Seismic damages of industrial process components and passive control techniques.

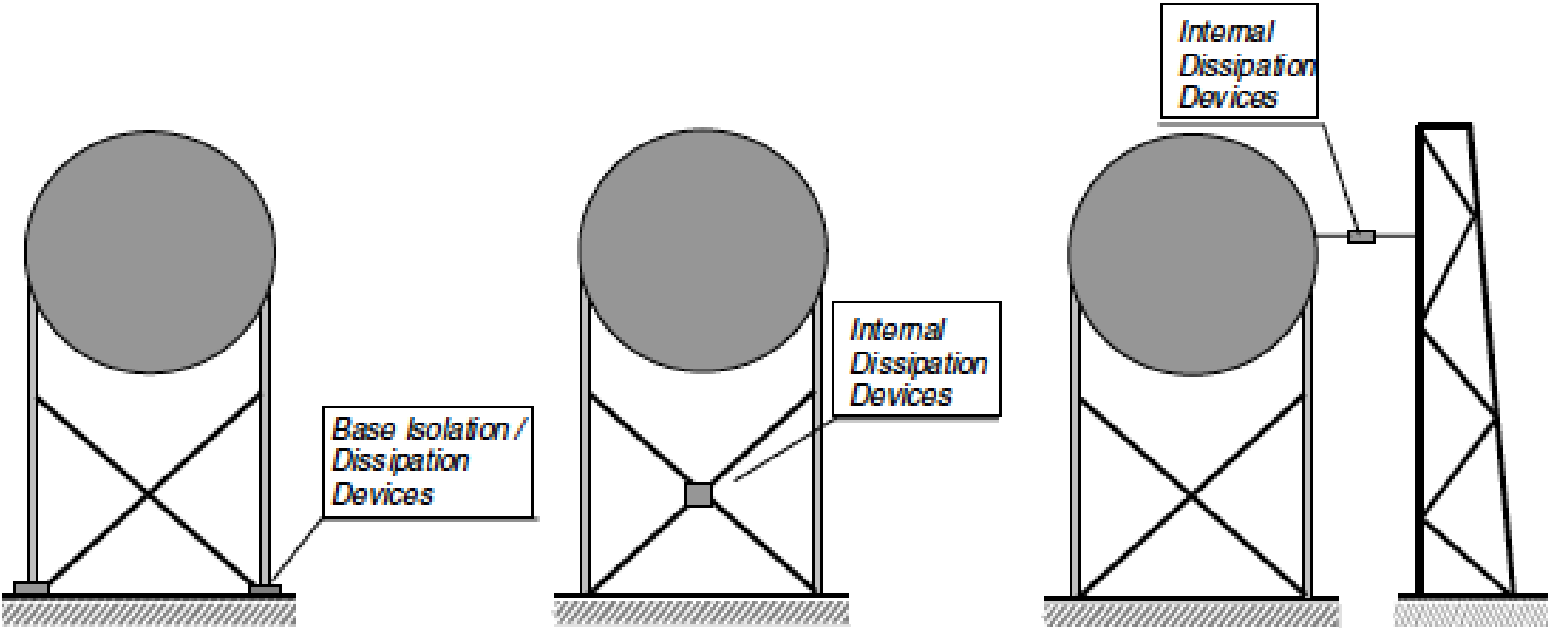
| Structural typology                     | Critical equipment                           | Typical seismic observed damages   | Other possible damages   | Passive control techniques   |
|---|--|--|--|--|
| Slim vessels                            | Columns<br>Reactors<br>Chimney<br>Torch      | <ul style="list-style-type: none"> <li>Leakage of fluid in flanged joints</li> <li>Yielding of anchor bars</li> </ul>  | Overtuming   | Dissipative coupling   |
| Above-ground squat equipment            | Big broad tanks with fixed and floating roof | Failure of wall-bottom plate welding<br>Elephant foot buckling<br>Diamond buckling of tank wall<br>Settlements of ground<br>Impact of floating roof to tank wall.  | Uplifting  | Base isolation   |
| Squat equipment placed on short columns | Spherical tanks                              | Collapse of structure due to shear failure of columns  |  | Dissipative spacers between roof and wall, TMD<br>Dissipative bracings<br>Base isolation<br>Dissipative coupling<br>Base isolation |
|   | Process Furnaces                             | Collapse of structure due to shear failure of columns<br>Collapse of the chimney<br>Detachment of internal pipes<br>Detachment of the internal refractory material | Leakage from pipes;<br><br>Increase of temperature of Furnace wall | Dissipative bracings<br><br>TMD  |
|   | Cryogenic tanks                              | Collapse of structure due to shear failure of columns  |  | Base isolation   |
| Piping systems and support structure    | Steel or R.C. frames                         | Collapse for excessive stresses  | Damages to supported equipment (pipes, tanks,...)                  | Dissipative bracings<br>Dissipative coupling<br>Non-conventional TMD   |

### Possible techniques...



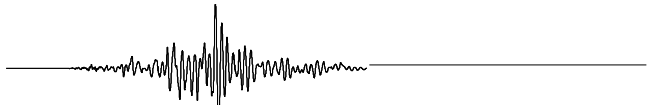
# Seismic protection of Industrial Components

## Passive Control Systems



**Figure 8: Different passive protection systems**

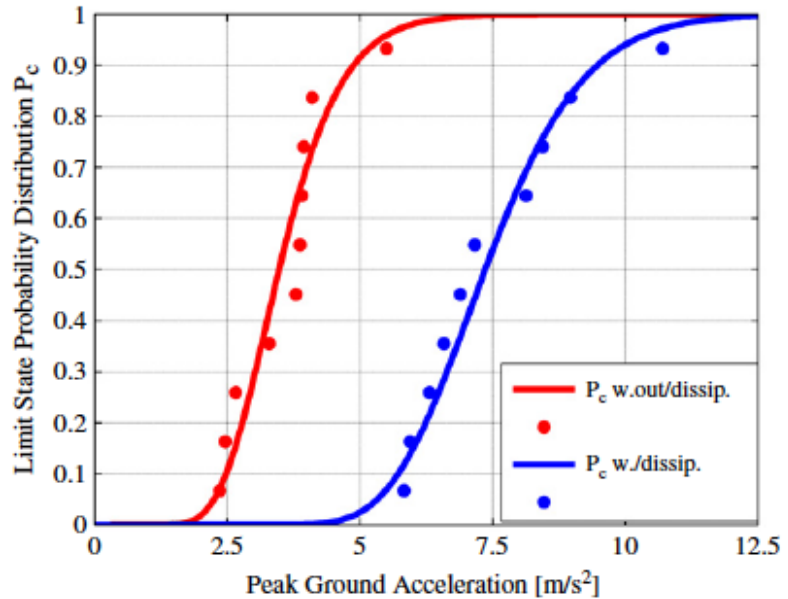
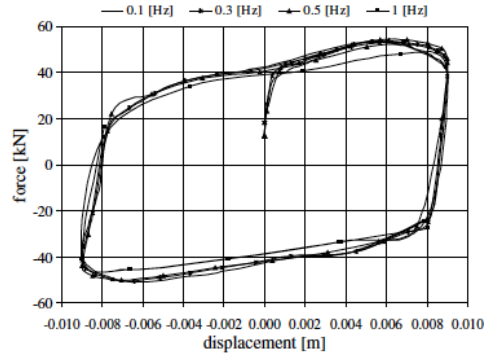
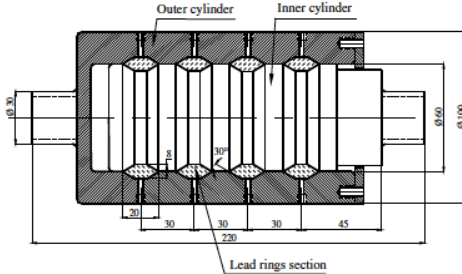
POSSIBILI SISTEMI DI PROTEZIONE PASSIVA PER SERBATOI



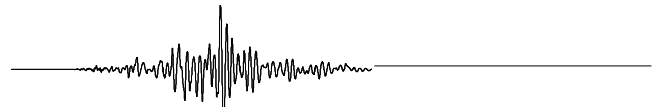
# Seismic protection of Industrial Components

## Passive Control Systems

### Dissipazione di energia (Curadelli 2011)



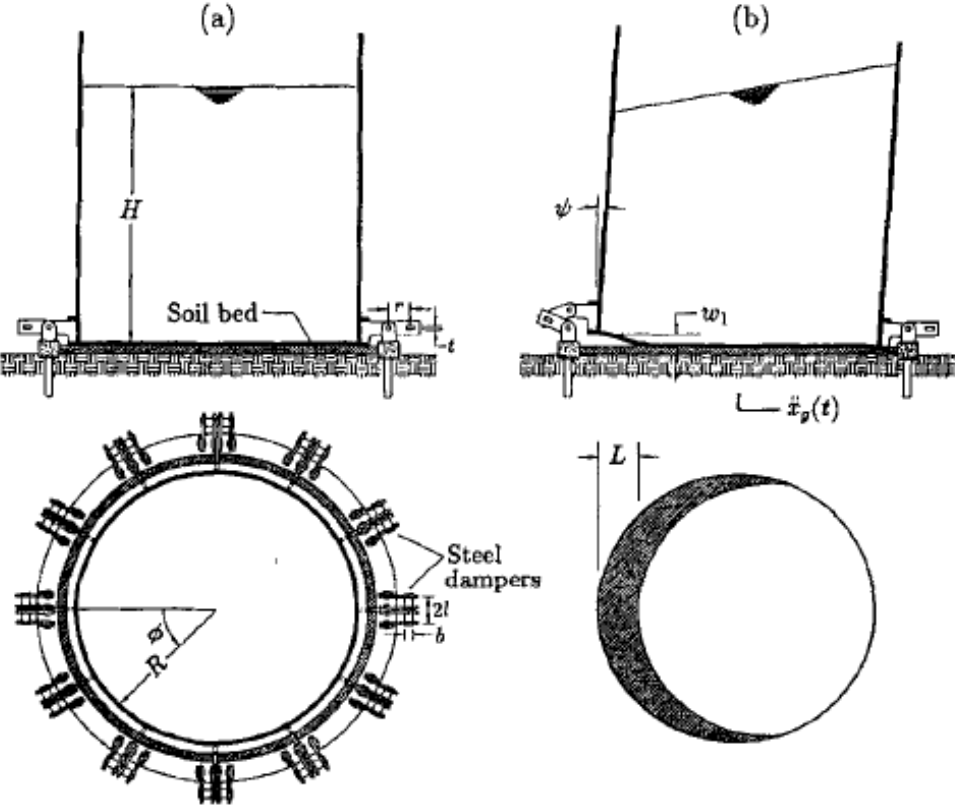
Controventamento dissipativo



# Seismic protection of Industrial Components

## Passive Control Systems

### Dissipazione di energia (Malhotra 1998)



N.B.  
Questa soluzione sfrutta il fenomeno dell'uplift, i cui spostamenti rispetto al terreno vengono sfruttati per attivare un meccanismo di dissipazione ad hoc

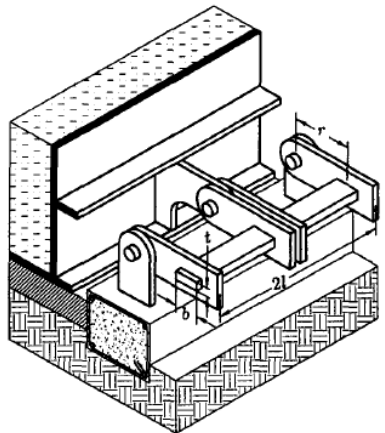
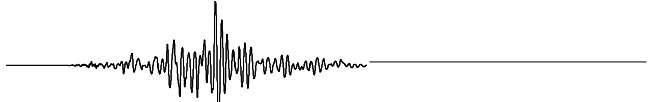


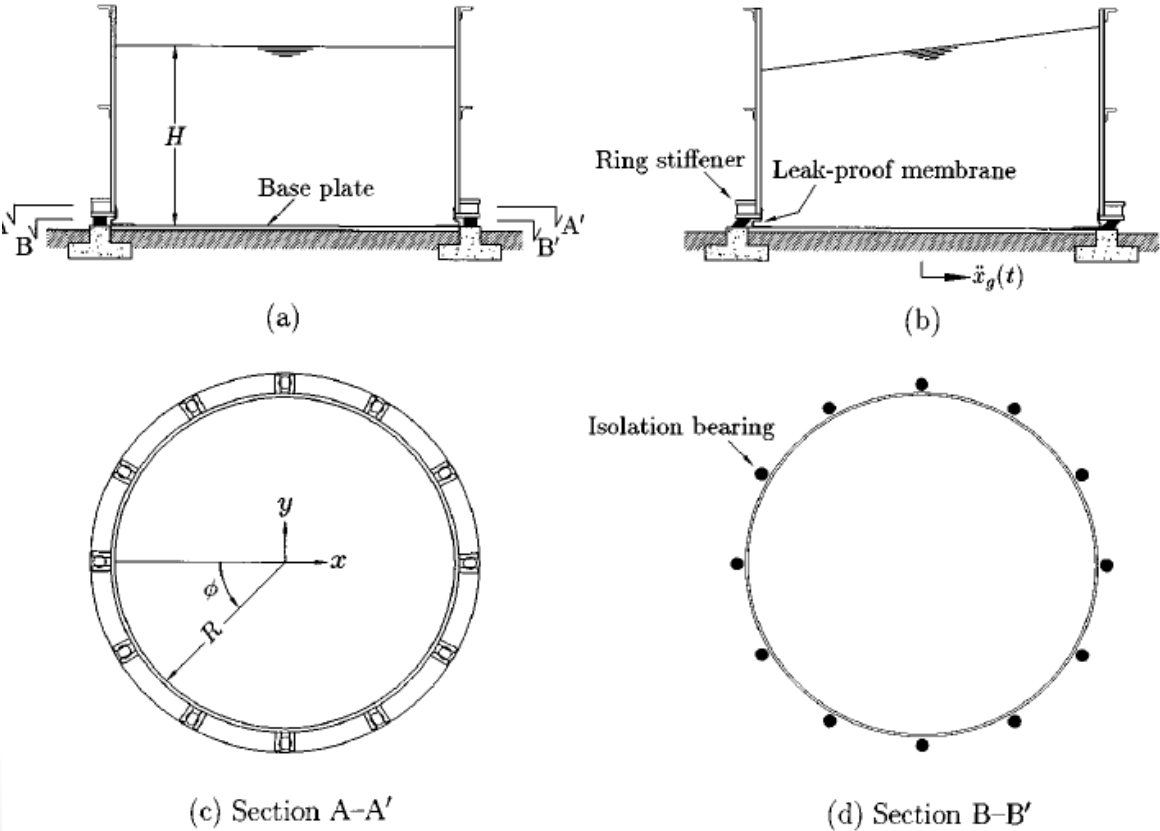
FIG. 1. Liquid-Storage Tank Anchored with Steel Hysteretic Dampers



# Seismic protection of Industrial Components

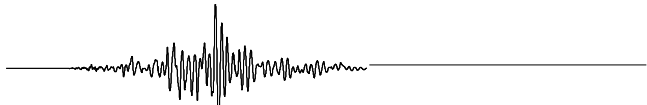
## Passive Control Systems

### Isolamento sismico (Malhotra 1997)



N.B.

Questa soluzione ha il vantaggio di non dover realizzare una soletta rigida alla base

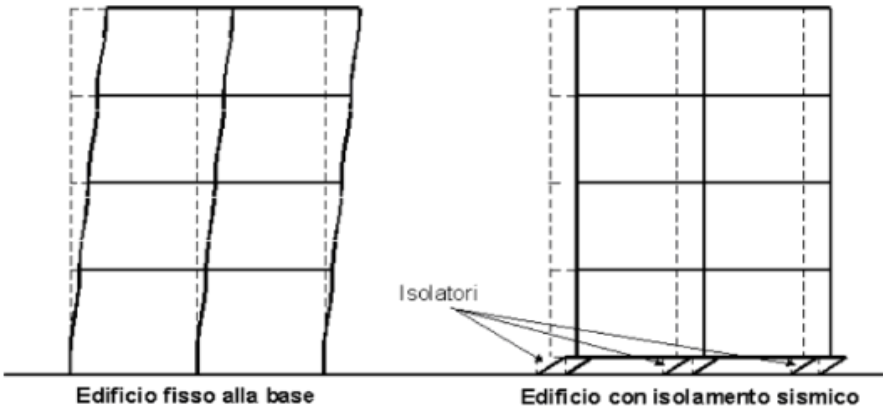




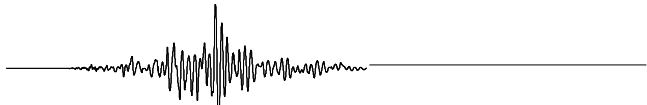
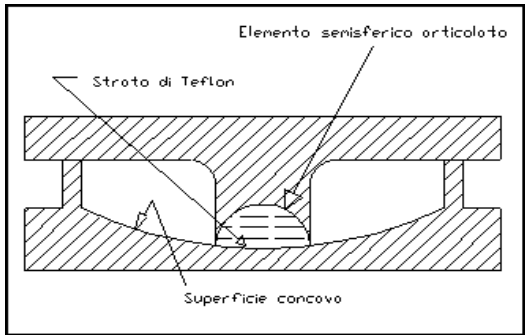
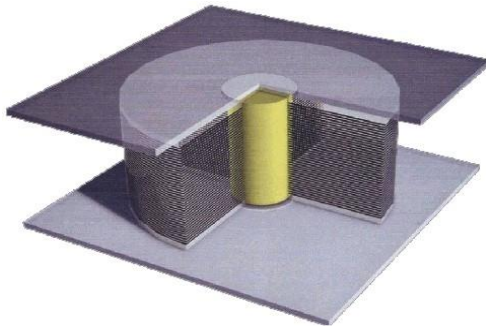
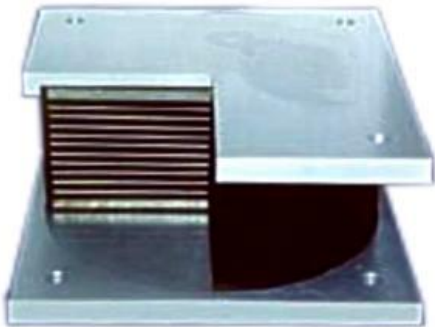
# Seismic protection of Industrial Components

## Base Isolation

Deformata, sotto l'azione di un terremoto, di un edificio tradizionale e di uno con isolamento sismico

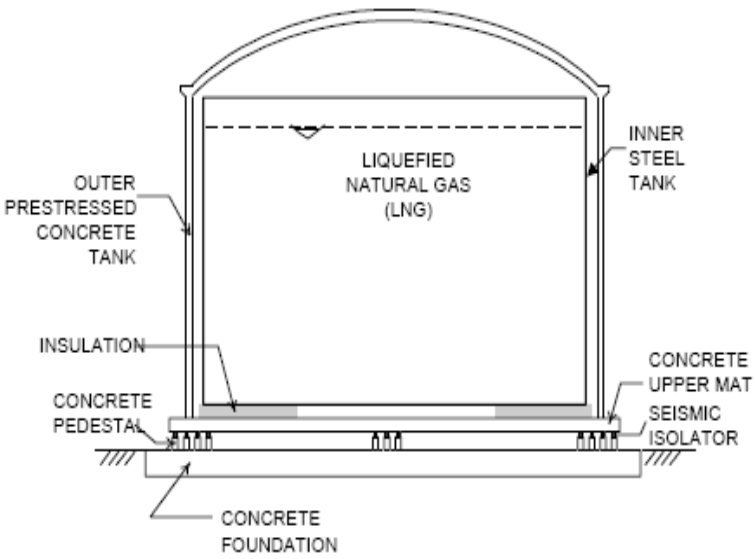
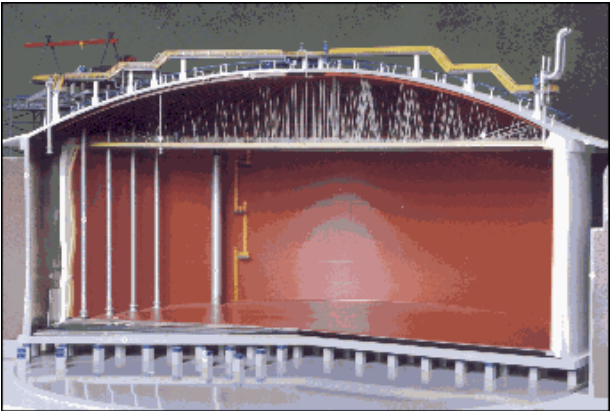
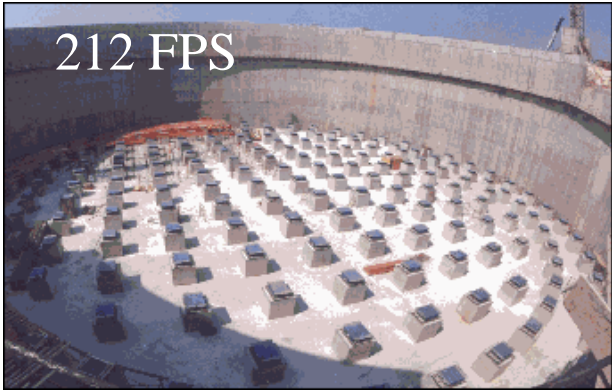


ISOLATORE ELASTOPLASTICO



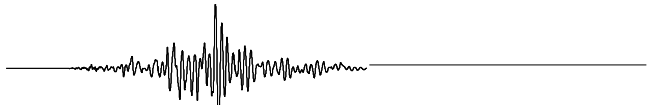
# Seismic protection of Industrial Components

## Base Isolation



Isolamento alla base di serbatoi LNG

Isolamento mediante FPS del serbatoio LNG di Revithoussa Island in Grecia



# Seismic protection of Industrial Components

## Base Isolation



## Isolamento alla base di serbatoi LNG

Isolamento mediante FPS del serbatoio LNG a Melchorita in Perù

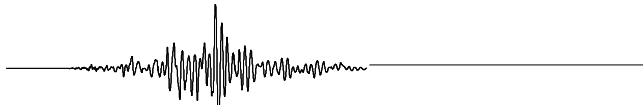


# Seismic protection of Industrial Components

## Base Isolation



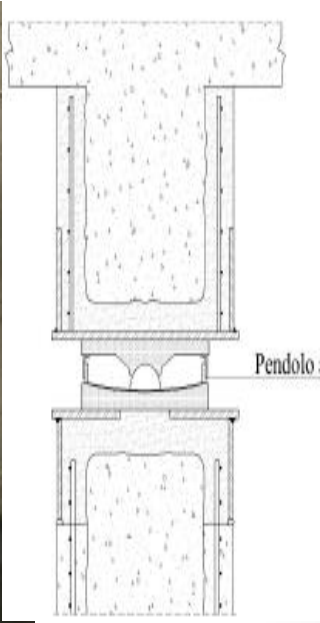
Seismic retrofitting of a steel tank using 26 HDRB isolators Switzerland



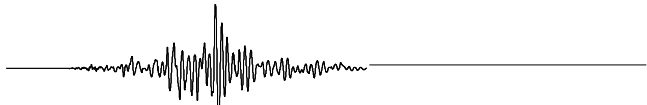


# Seismic protection of Industrial Components

## Base Isolation



Seismic retrofitting of a steel tank using FPS  
Petrolchemical pole of Siracusa  
Priolo Gargallo (Sr) – Sicily



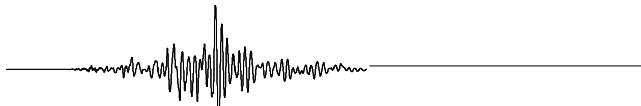


# Seismic protection of Industrial Components

## Base Isolation

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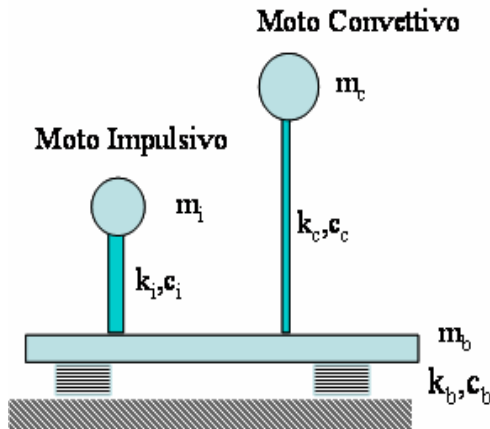
Quali tipi di isolatori utilizzare?



# Seismic protection of Industrial Components

## Base Isolation

- Si può utilizzare il modello a pochi gradi di libertà
- Il comportamento non lineare dei dispositivi di isolamento può essere agevolmente implementato
- Per un progetto di prima approssimazione si può ipotizzare che la massa isolata sia solo quella impulsiva, essendo quella convettiva già naturalmente isolata avendo un periodo molto elevato.
- Di conseguenza la rigidezza del sistema di isolamento può essere facilmente ricavata dalla relazione seguente

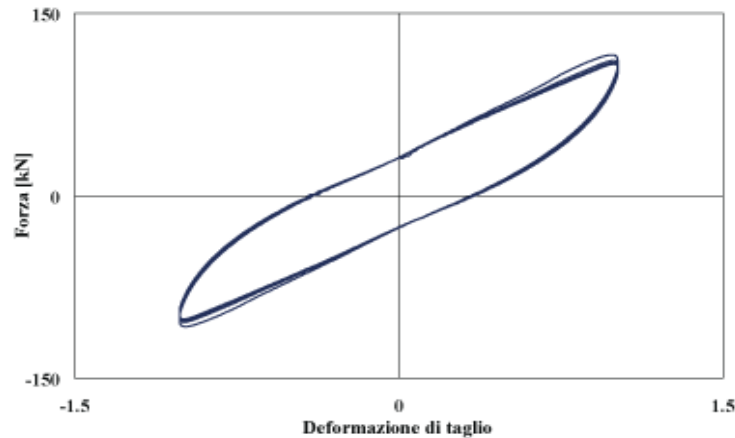


$$T_{iso} \approx 2\pi \sqrt{\frac{m_i + m_s + m_b + m_w}{k_{iso}}} = 2\pi \sqrt{\frac{m_{tot}}{k_{iso}}}$$

**Periodo del serbatoio isolato**

# Seismic protection of Industrial Components

## Base Isolation



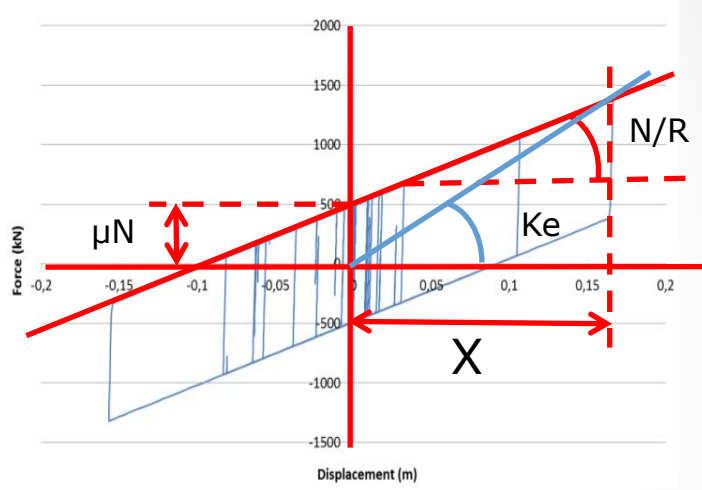
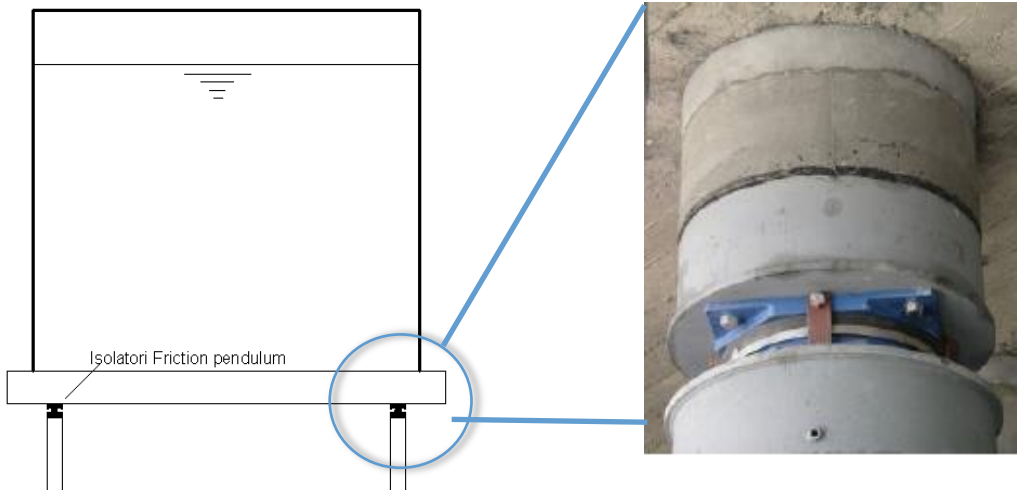
### PROCEDURA DI PROGETTO:

- 1) Si ipotizza il periodo di isolamento  $T_{iso}$
- 2) Individuata la massa da isolare si calcola la rigidezza  $K_{iso}$
- 3) Ipotizzato uno smorzamento (10%) si valuta dallo spettro di risposta degli spostamenti, lo spostamento massimo dell'isolatore
- 4) Ipotizzando una deformazione del 100% si calcola l'altezza della gomma
- 5) Si determina il numero degli isolatori in maniera da rispettare le condizioni di sicurezza nei confronti dell'instabilità



# Seismic protection of Industrial Components

## Base Isolation

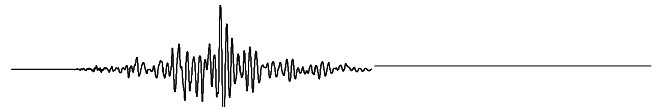


$$T_p = 2\pi \sqrt{\frac{m}{k_e}} = 2\pi \sqrt{\frac{m}{g m \left(\frac{1}{R} + \frac{\mu}{X}\right)}} = 2\pi \sqrt{\frac{l}{g \left(\frac{1}{R} + \frac{\mu}{X}\right)}}$$

Vibration Period of base-isolated structure with FPS

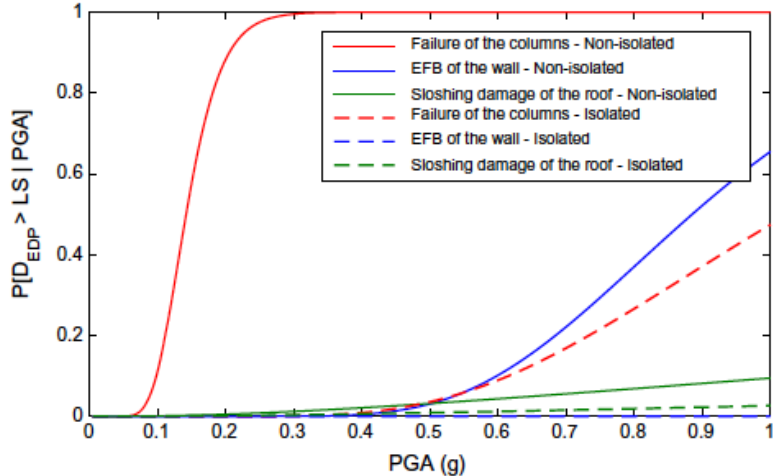
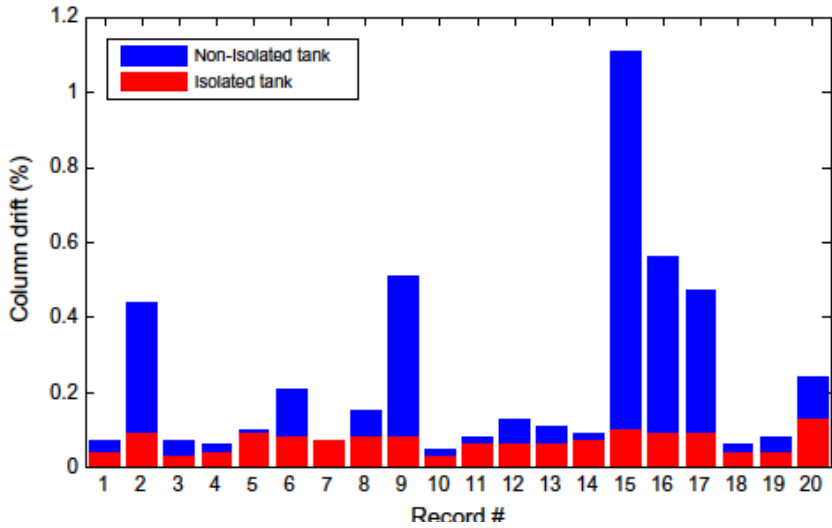
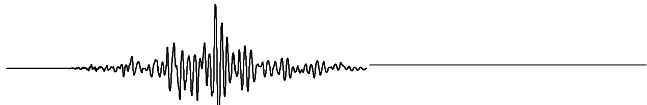
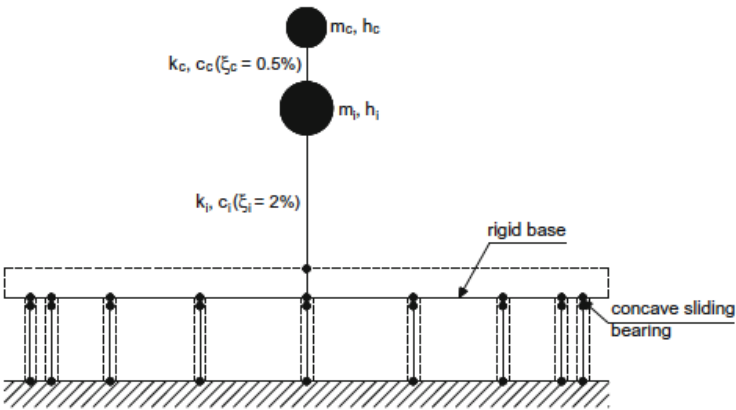
$$T_p = 2\pi \sqrt{\frac{m}{k_e}} = 2\pi \sqrt{\frac{m_{imp} + m_{ss} + m_{iso}}{g m_{tot} \left(\frac{1}{R} + \frac{\mu}{X}\right)}}$$

Vibration Period of base-isolated Tank with FPS



# Seismic protection of Industrial Components

## Base Isolation: Example





# Conclusions (**Riscrivere**)

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- The need to study carefully the earthquakes effects on industrial plants with major-accident hazards has been addressed.
- In particular, typical equipment and components of refineries were identified, their vulnerability was analyzed, both looking for into historical events, concerning damages caused by past earthquakes to several industrial plants and investigating the typology of the structural nature of the different components of a plant.
- This analysis allowed identifying the most critical elements both for their seismic vulnerability and for the consequences of their damage state
- Vulnerability assessment methods based on fragility analysis of storage tanks has been illustrated



# Conclusions

---

- The application of Fragility analysis has been illustrated on an emblematic case study of an actually collapsed LNG tanks
- Nonlinear time history and probability response analyses using a 3D model have been presented.
- Cloud and IDA approaches for data collection and statistically appropriate methods for fragility function fitting were discussed.
- The Cloud Method involves the non-linear analysis of the structure subjected to a set of un-scaled ground motion time-histories.
- IDA can be quite time-consuming as the non-linear dynamic analyses using scaled ground motion time-histories for increasing levels of ground motion intensity.



# Conclusions

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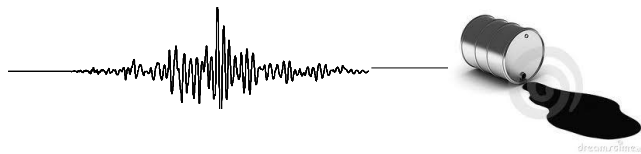
- Supporting columns were collapsed by pure-shear and investigated by drift ratio at shear failure.
- Tank shell wall buckling and liquid sloshing also investigated and compared with the capacities.
- It was evidenced that failure in support structure columns is the most influencing one. This is fully in accordance with the real collapse mode.
- The application of the base isolation technique appears particularly effective in reducing the impulsive pressure on the tank wall and then of the corresponding base shear.
- An application on elevated tanks demonstrated this assertion





Thank you very much for your attention

Questions?



**Novembre 17 2016, Bologna**